



CIVIL ENGINEERING & INFRASTRUCTURE ASSESSMENT REPORT

Planning Proposal for:

93 BRIDGE STREET WESTMEAD, NSW

Prepared for:

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1 INTRODUCTION & SCOPE

1.1 Introduction & Scope

Costin Roe Consulting Pty Ltd has been commissioned by 93 Bridge Road Pty Ltd atf Bridge Road Unit Trust, to undertake a site conditions and infrastructure services assessment to assist in the Planning Application for a proposed redevelopment of the land at 93 Bridge Road, Westmead, NSW.

The proposed redevelopment of the site, for residential or mixed-use buildings, has Floor Space Ratio (FSR) and heights which exceed the current provisions of *Parramatta Local Environmental Plan 2023* (PLEP2023).

The information provided in this report is intended to inform the Master Planners of the opportunities and constraints associated with the civil engineering requirements and for the provision of infrastructure services to the site for the proposed redevelopment. The purpose of the report is also to consider the Stormwater Management for the property and to confirm a solution to meet Parramatta City Councils specific stormwater management objectives for stormwater quality and quantity.

Specifically, this report provides the following information relating to services:

- Location and indicative capacity of existing service networks for Potable Water (drinking water), Waste Water (sewer), Power, Natural Gas; and Telecommunications;
- Indicative utility demands for the current development proposals where available;
- Current service infrastructure delivery programs from the primary utility suppliers where available; and
- Service infrastructure assets required onsite.

Civil engineering, stormwater management and flood planning considerations include:

- Management of stormwater quantity (on-site detention) and quality;
- Flood planning considerations including that relating to Parramatta River;
- External Stormwater Catchments and Overland Flow.

The site is located within the bounds of the City of Parramatta Council (CPC). City of Paramatta Council policy applies and has been used in the framework of this document.

This report should be read in conjunction with the *Planning Proposal Report* prepared by Willow Tree Planning which sets out the relevant strategic case for the increased FRS and building heights.

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1.2 Site Description

The 93 Bridge Road Site (SP 31901) is located on the eastern side of Bridge Road in the Suburb of Westmead as shown in **Figure 1.1**.



Figure 1.1 Locality Plan (Source: SIX Maps 2023)

The property comprises an area of 0.87 Ha and is bound by Bridge Road on the west, existing residential town house development to the north, and residential development at 91 Bridge Road known as the "Reveria Park Monarco Estate" on the east and south.

The site is currently developed, with a series of residential townhouse improvements over the land. The land form generally falls from west to east with the existing townhouse levels gradually stepping down also from west to east to fit the landform of the site. Site levels vary between RL 29.3m AHD at on the south-west corner of the site (at the intersection of Bridge Road and Grande Corniche) to RL 23.0m AHD at the north-eastern corner of the site. Over the length of the site, the falls are in the order of 5-6% on average.

A shared access, "Grande Cornich", is located on the south of the development and serves both the 91 and 93 Bridge Road properties.



Council drainage line (600mm dia.), and associated 3.0m wide easement, conveys stormwater from site from the south-east corner of the property. The pipe and easement then head in an east and northerly direction to a local gully and ultimately to the Upper Parramatta River which is approximately 700m north of the subject property.

A geotechnical investigation is not available at the time of writing. Review of the Penrith Geological 1:100000 Series Sheet however shows the site to be underlain by Ashfield Shale of the Wianamatta Group. These shales will usually be within 1-2m of the natural ground surface and will generally comprise weathered to highly weathered claystone-siltstone composition. The shale will normally be excavatable by ripping or other normal earthworks machinery and will become harder as excavation becomes deeper and the material is less weathered. This will need to be confirmed by investigation – there will be a trade-off between excavatability and ground support during excavation for the basement levels which would be considered as part of a normal detail design exercise. The shale bedrock is expected to be overlain soils comprised of residual clays, with depths typically in the order of 1-2m as noted.

1.3 Proposed Application

The proposed application is for an increase to the FSR and building heights referenced in the PLEP2023 for a range of residential and commercial uses. An indicative Masterplan Layout of the development site has been produced by RobertsDay which takes into consideration economic, social and engineering opportunities and constraints.

The proposed indicative built form layout is shown in Figure 1.2 below.



Figure 1.2 Indicative Built Form Masterplan Layout



The indicative built-form layout considers urban integration of public domain with surrounding infrastructure and intends to provide a space which invites people to live, work and play in its environment.

The current expected target mix and yield for the development is approximated as follows:

APARTMENTS

Market Residential (411 Dwellings) – 31,070 sqm Affordable Residential (75 Dwellings) – 5,480 sqm

SUPPORTIVE

Retail - 260 sqm

SITE AREA - 8,663 sqm

TOTAL GFA - 36,810 sqm

COMBINED FSR - 4.25:1

As these target mixes and yields are preliminary and subject to adjustments, for the purpose of this report and following services capacity and demand assessments we have adopted the following in our assessments, and rounded to provide a conservative estimate of demands as included in **Section 2** of this report:

- Residential 40,000 m²
- Commercial 300 m²



2 INFRASTRUCTURE SERVICES

An overview of the existing and proposed infrastructure network layouts are outlined in the following sections. Reference to **Appendix C** should be made for Dial-Before-You-Dig information for each service.

Demand has been estimated based on the nominal yield rates included in **Section 1.3** of this report. As noted, these rates provide a conservative estimate of demand based on typically adopted demand rates for different uses.

2.1 Potable Water (drinking water)

Sydney Water is the servicing authority for potable water in the suburb of Westmead.

A DN375mm DICL is also located on the eastern side of Bridge Road adjacent to the western property boundary. An existing DN100mm DICL main is also present on Grande Cornich on the southern property boundary. Existing connections to 93 and 91 Bridge Road properties appear to be made from this smaller main.

A DN100mm CICL main is also present on the western side of Bridge Road.

Refer Appendix C.

<u>Potable Water Demand</u>

Typical water demand rates for different land uses are provided in **Table 2.1** below.

Land Use	Design Criteria	Units	Potable Water Demand
Single Dwelling Residential (14 - 17 dwelling/net ha)	Max Day Demand	kL/dwelling/day	2.2
Town House (<30 units/net ha)	Max Day Demand	kL/unit/day	1.6
Multi/ high Rise Units	Max Day Demand	kL/N Floor Ha/day	33.5
Light Industrial	Max Day Demand	kL/N Ha/day	40
Medium Industrial	Max Day Demand	kL/N Ha/day	66
Suburban Commercial	Max Day Demand	kL/N Ha/day	41
City Rise Commercial/ shopping Centre	Max Daily Demand	kL/floor Ha/day	63

Table 2.1 Water Demand Unit Rates

Utilising the multi-unit rate of 33.5kL/N Floor Ha/day over net residential development, and City Rise Commercial of 63 kL/N Floor Ha/day for retail and medical suites, a demand for the development can be calculated.

A demand in the order of 136 kL/day is expected to be met to service the indicative built form layout.



Potable Water Capacity

In order to gain an understanding of the capacity of the existing water mains to service the development, the empirical guide to pipe servicing capacity contained in the Water Supply Code of Australia (Sydney Water Version) was used. This is shown in **Table 2.2** below.

Nominal Size of Main DN			Capacity of Main (single direction feed)			
DICL PN36	PVC PN16	PE PN16 PE80B & PE100	Residential (Lots – 500m²)	Rural Residential (lots)	General/ Light Industrial (ha)	High Usage Industrial (ha)
100	100	125	40	-	-	-
150	150	180	160	125	23	-
200	200	250	400	290	52	10
250	250	315	650	470	84	24
300	300	-	1000	-	-	-
375	375	-	1650	-	-	-

Table 2.2 Empirical Sizing of Reticulation Mains

Using the empirical guidelines, the DN375mm water main in Bridge Road would have a capacity to service approximately 1650 apartments or dwellings.

It is noted the current information does not enable us to estimate the extent of other properties being serviced by the water main and further investigations will be required in this regard. Confirmation of the capacity of the existing system will need to be confirmed via a Section 73 Application to Sydney Water performed by a Sydney Water qualified Quick Check agent. As part of this assessment specific volumetric rates and water pressures will be able to be provided by Sydney Water and assessed by a qualified hydraulic engineer.

Potable Water Supply

The strategy and design for the required extension of the system will need to be performed by a Sydney Water Service Coordinator.

A qualified Hydraulic Engineer shall design internal water and fire system water supply to service the proposed development sites. This will be investigated as part of the detailed design and assessed as part of future separate building development applications.

Given the location of the development which is within a substantially developed urban area, and the major water mains present in Bridge Road, it is expected that infrastructure of sufficient capacity is available and accessible in the required timeframes for the development of the land.



Notwithstanding the further investigations and applications required with Sydney Water, it is considered that water supply will be able to be provided to the development site.

It should also be noted that rainwater reuse will be employed on this development for non-potable applications including toilet flushing and irrigation which will significantly reduce the above noted demand requirements.

2.2 Waste Water (sewer)

Sydney Water is the servicing authority for sewage disposal in the suburb of Westmead.

Wastewater infrastructure is located in the immediate vicinity of the development site in the form of a 150VC, 225mm DICL and 150mm uPVC sewer mains. The existing site connection is via the 150mm VC main located at the south-east corner of the site. The sewer main is shown to drain in an easterly direction from the site.

Waste Water Service Demand

The design criteria used to forecast future sewer loadings are generally taken from the *Sydney Water Area Planning Design Criteria Guide* and are expressed as an Equivalent Population for a particular land use. The Average Dry Weather Flow (ADWF) per Equivalent Population (EP) is taken as 180 L/day or 0.0021 L/s (ADWF (L/s) = 0.0021 x EP). Alternatively an estimate of the ADWF can be made based on 80% of the expected potable water demand.

Values for typica	l deve	lopment types ar	e summarised ir	n Table 2.3 .
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Item	Units	Adopted Value	Source
Single Dwelling Residential	EP/dwelling	3.5	SWC Area Planning
Medium Density Residential	EP/dwelling	3.0	Design Criteria Guide
(townhouses up to 4 storeys)			
High Density Unit	EP/Bedroom	0.275	
Development (up to 200-400			
Bedrooms/ Ha)			
Light Industrial	EP/ha	75	
Heavy Industrial	EP/ha	150	
Commercial	EP/ha	75	
High Density Commercial	EP/ha	300 - 800	WSA 02-2002-2.2
Reserves	EP/ha	20	SWC Area Planning
			Design Criteria Guide

An estimate of the Average Dry Weather Flow (ADWF) for the project has been calculated based on 80% of the expected 136 kL/day demand. An ADWF of 109 kL/day or 1.26 L/s has been estimated for the development.

Note that the design of sewer mains will apply a peaking factor to the ADWF to get the Peak Daily Dry Weather Flow (PDWF), and include the peak (rainfall dependent) inflow and infiltration and the groundwater (non-rainfall) dependent infiltration. The peaking



factor will vary depending on the size of the upstream catchment and would normally be within a range of 2 to 5, i.e. 2.52 - 6.3 L/s.

Waste Water Capacity

The existing DN150 main on the eastern boundary of the site is expected to have a capacity in the order of 10-12 l/s.

Waste Water Removal

Connection of the site waste water can be made into the existing DN150 mains on the western boundary of the site. The Sydney Water DBYD plan suggests several existing tap in points are available.

Given the capacity of the connecting main is above the required output from the development it is expected that the existing main will be sufficient to cater for the development. The extent of the upstream catchment being serviced by the main however is not known and confirmation of the proposed strategy will need to be performed in conjunction with Sydney Water via a Sydney Water qualified Water Service Coordinator during project application stage.

Notwithstanding the further investigations and applications required with Sydney Water, it is considered that waste water reticulation will be able to be provided to the development site.

2.3 Power

The DBYD information shows there are Endeavour Energy duct routes running along the eastern sides of Bridge Street and southern side of Grande Cornich, and extending into the site boundary. A substation (PM SUB 17293) is located at the south-eastern corner of the property and appears to service the 91 and 93 Bridge Road properties.

The details, voltage and capacity of the above mentioned cables are however unclear. We would expect that the lines running through the site will be made redundant during future works. Further checks and confirmation would need to be carried out by an accredited electrical consultant during the detailed design stage.

Power Demand

A high level estimate of the power demand for the development has been estimated using average values for different land uses typically adopted by Integral Energy. These rates are generally considered to be reasonably conservative but are appropriate for preliminary estimation purposes.

Table 2.4 presents the unit rates applied to the power usage calculation and the estimated total power usage for the development.

Land Use	Rate (kW/unit/day)
Residential Dwelling (per dwelling)	5
Light Industrial, Employment, Town Centre, Village Centres,	0.04
Schools (per m ²)	

Table 2.4 Preliminary Power Usage Estimate



To give a reasonable estimate of the daily usage for a multi-unit development such as the proposed development, a diversity factor should be applied to the usage estimate to make allowance for the variation in usage across the development and the difference between a residential dwelling and unit. For this calculation a typical diversity factor of 0.5 to 0.8 is applied to the sum of the total power usage presented in **Table 2.4**, giving a maximum demand estimate for the Precinct of approximately 1.3 - 2.1 MW per day for a total of 600 apartments.

Power Capacity and Supply

In order to confirm capacity, further investigation and applications will be required with Endeavour Energy by a qualified electrical consultant. It is noted that the new development would have significantly higher demands than the existing townhouse development and some amplification could be required to suite the expected demand range of the development.

Notwithstanding the further investigations and applications required with Endeavour Energy, it is considered that power supply will be able to be provided to the development site.

2.4 Natural Gas

Jemena is the servicing authority for gas supply adjacent to the site.

Gas infrastructure is available in the form of pressure mains with a 210kPa capacity running along the southern side of Grande Cornich and the eastern side of Bridge Road.

Natural Gas Demand

Jemena uses an energy demand of 20 gigajoules per year to estimate the average annual domestic usage of natural gas for residential dwellings. This usage rate typically equates to the utilisation of a natural gas hot water tank, cook top and heating point.

Table 2.5 presents the natural gas usage estimate for the project, assuming supply to residential units only. This estimate uses a conversion factor of 39.6 m³ / GJ to convert the estimated energy usage into a volume of gas (Roarty, 2008).

Land Use	Number of Dwellings/ Units	Rate (GJ/dwelling/yr)	Conversion Rate (m3/GJ)	Usage (GJ/yr)	Usage (GJ/day)	Usage (m³/day)
Residential Dwellings	600	20	39.52	11895	32.6	1,298

Table 2.5 Natural Gas Usage Estimate

The estimated natural gas usage for 600 residential apartments is approximately 1,298m³/day.

It should be noted that this natural gas demand estimate excludes demands from commercial premises. Without details of the specific proposed land uses it is difficult to



estimate a gas demand for these areas. To meet BASIX requirements, it is expected that the residential units will require a natural gas connection.

Natural Gas Capacity and Supply

The existing DN250 main will have a carrying capacity in the order of 100-150m³/hour or 2,400-3,600m³/day. Confirmation of sufficient capacity for gas supply will be required by a specific application to Jemena.

Notwithstanding the further investigations and applications required with Jemena, it is considered that gas supply will be able to be provided to the development site if required. We would expect this to be on an as- needed basis for individual buildings.

2.5 Telecommunications

At the time of writing we have not received a response from NBN Co or other telecommunication providers including Telstra.

We expect that the existing local cable network would not have the capacity to service the proposed development and that new underground cabling would be required to suit the project requirements. Normally this would be completed on a project by project basis.

Demand and capacity calculations have not been included in this report for telecommunications as these are not readily quantifiable like the other services assessed. The requirements for telecommunications would need to be formalised via a Telstra Smart Community (or similar) registration.

Notwithstanding the further investigations and applications required with Telstra, it is considered that telecommunication infrastructure will be able to be provided to the development site.



3 EROSION AND SEDIMENT CONTROL

3.1 Background

Construction of the project would require demolition and extensive excavation for basement levels and foundations.

During the construction phase of the development, an *Erosion and Sediment Control Program* will be implemented to minimise water quality impacts. A detailed *Erosion and Sediment Control Program* will be employed throughout the construction works and a concept for this will be defined during the Development Application stage of the development site. The *Erosion and Sediment Control Program* will be defined based on normal engineering guidelines including The Landcom publication, *Managing Urban Stormwater: Soils and Construction (The Blue Book)* and the requirements of Parramatta City Council. It is expected that the program will include measure such as temporary sediment basins, silt fences, cut-off drains for polluted stormwater and diversion channels for clean stormwater run-off.

The following sections provide information to identify controls and procedures that will be incorporated into the *Erosion and Sediment Control Program* at Development Application Stage.

3.2 Pre-Construction

The following minimum requirements are to be met prior to commencement of construction:

- Protection of downstream receiving waters. The proximity to Parramatta River will require additional considerations to ensure that receiving waters are protected.
- Sediment fences are to be constructed on the upstream edges of the designated buffer strips and at the base of fill embankments.
- Areas for plant and construction material storage are to be designated along with associated drains and spillage holding ponds.
- Diversion banks are to be created at the upstream boundaries of construction activities to ensure upstream runoff is diverted around any exposed areas.
 Catch drains are to be created at the downstream boundary of construction activities.
- Silt fences and/or sand bags are to be placed along the catch drains to slow flow, reduce scour and capture some sediment from runoff.
- Construction of temporary sediment basins.



• Site personnel are to be educated to the sediment and erosion control measures implemented on site.

3.3 During Construction

The following minimum requirements are to be met during construction:

- Progressive re-vegetation of filled areas and filled batters.
- Construction activities are to be confined to the necessary construction areas.
- The provision of a construction exit (truck shaker) to minimise the tracking of debris from tyres of vehicles leaving the site onto public roads. Only one construction exit will be nominated to limit the movement of construction equipment.
- Topsoil and temporary stockpile location will be nominated to coincide with areas already disturbed. A sediment fence is to be constructed around the downstream side of the stockpile and a diversion drain at the upstream side if required.
- Regular inspection and maintenance of silt fences, sediment basins and other
 erosion control measures are to be made. These should be undertaken
 weekly, monthly and following major rainfall events. Following rainfall events
 greater than 50mm inspection of erosion control measures and removal of
 collected material should be undertaken. Replacement of any damaged
 measures should be performed immediately.

3.4 Post Construction

The following minimum requirements are to be met post construction:

- The contractor/ developer will be responsible for the maintenance of erosion and sediment control devices from the possession of the site until the site is accepted "Off Maintenance" or until stabilisation has occurred to the satisfaction of the superintendent and council.
- Key stormwater areas requiring maintenance for operational phase of the project following construction are piped stormwater system, bio-retention areas, field inlet pit inserts and rainwater tanks.



4 STORMWATER DRAINAGE

4.1 Site Drainage

4.1.1 Existing Drainage System

As part of the existing residential development on the property, an in-ground drainage system is present. This system comprises grated inlet pits, sealed junction pits, down pipe connections and in-ground pipes which convey stormwater from buildings, car parks and circulation areas to the legal point of discharge at the south-east corner of the site and Parramatta River.

4.1.2 Proposed Drainage System

As per general engineering practice and the guidelines of City of Parramatta Council, the proposed stormwater drainage system for the development will comprise a minor and major system to safely and efficiently convey collected stormwater run-off from the development to the legal point of discharge. Details and layout of the proposed system for the development will be defined during the Development Application Stage of the project.

The minor system will consist of a piped drainage system designed to accommodate the 1 in 20-year ARI storm event (Q20). This results in the piped system being able to convey all stormwater runoff up to and including the Q20 event. The major system will be designed to cater for storms up to and including the 1 in 100-year ARI storm event (Q100). This major system employs overland flow paths to safely convey excess run-off from the site.

The future design of the stormwater system for the site should be based on the following documents:

- Runoff from buildings will generally be designed in accordance with AS 3500.3
 National Plumbing and Drainage Code Part 3 Stormwater Drainage.
- Overall site runoff and stormwater management will generally be designed in accordance with the Institution of Engineers, Australia publication "Australian Rainfall and Runoff" (1988 Edition), Volumes 1 and 2 (AR&R).
- Design recurrence intervals for major and minor storms will be in accordance with Part 5 of CPC Development Control Plan 2023.
- On-site detention, water quality measures and flooding requirements will be in accordance with Version 4 of the Upper Parramatta River Catchment Trust On-site Detention Handbook; and
- Stormwater harvesting is based on the requirement of CPC Development Engineering Design Guidelines 2018 and the NSW Department of Environment and Conservation document *Managing Urban Stormwater: Harvesting and Reuse*.

Water quality and re-use are to be considered in the design, throughout new paved areas, to ensure that any increase in stormwater pollution are mitigated, Councils Water Quality Objectives are met and that the demand on potable water resources is reduced.



The means to which these objectives are achieved can broadly be summaries as follows:

Water Quantity

City of Parramatta Council requires water quantity management, or stormwater detention, to be provided to limit the runoff discharged from private property into the underground piped drainage system to pre-developed flow and to assist in mitigating the increased stormwater runoff generated by development.

On-site detention is not proposed as part of the current development application for subdivision and infrastructure works for the subdivision. On-site detention will be required to be provided for individual developments once development of these land parcels takes place. The purpose of this section of the report is to define the requirements for these future developments, taking into account the offset requirements for the un-attenuated road catchment.

The site is contained within the Upper Parramatta River Catchment and as per CPC requirements calculation of the site storage requirement (SSR), i.e. the detention storage, and permissible site discharge (PSD) is required to be made using Version 4 of the Upper Parramatta River Catchment Trust On-site Detention guidelines. The basic PSD and SSR rates are shown in **Table 4.1**. below:

Basic OSD Requirements		% impervious surface	
		100%	85-100%
PSD (l/s/ha)	PSD (I/s/ha) Primary Outlet Secondary Outlet		As per UPRCT OSD
			Guidelines
Provided	Primary Outlet	34	
PSD (I/s)	PSD (I/s) Secondary Outlet		
SSR (m³/ha)		455	
Storage Requ	ired for 0.86 Ha (m³)	392	

Table 4.1. Basic Detention SSR and PSD Requirements and detention size

A 392m³ detention system would be required for the development site, reference to drawing **Co13829.01-SK01** shows a possible arrangement for the system based on an underground detention system.



Water Quality

The City of Parramatta have nominated, in Part 5.1 of Council's DCP 2023, the requirements for stormwater quality to be performed on a catchment wide basis. These are presented in terms of annual percentage pollutant reductions on a developed catchment and are as follows:

Gross Pollutants	90%
Total Suspended Solids	85%
Total Phosphorus	65%
Total Nitrogen	45%
Total Hydrocarbons,	90%
Motor Oil and Grease	

Roof, hardstand, car parking, roads and other extensive paved areas are required to be treated by the Stormwater Treatment Measures (STM's). The STM's shall be sized according to the whole catchment area of the development. The STM's for the development shall be based on a treatment train approach to ensure that all of the objectives above are met in line with the principles of Water Sensitive Urban Design (WSUD) considerations.

Stormwater quality objectives will be met via a treatment train which comprises a combination of proprietary and WSUD measures. A treatment solution, whereby a bio-retention filtration system (approximately $100m^2$) could function as the tertiary treatment measure has been provided in the planning application to show that a solution is available for the development. Pre-treatment (or primary treatment) of litter, coarse sediments, and hydrocarbons would be necessary to ensure early onset sedimentation of a bio-retention system did not occur if this is the final adopted water quality measure.

It is noted that proprietary filtration systems could be adopted to address water quality objectives and to meet the required pollution reduction percentages. Typical systems provided by water quality manufacturers such as Humes, Rocla, Ocean Protect and Atlan (SPEL) could be considered for the development.

Reference to **Appendix B** shows the indicative location and configuration of the stormwater management plan for the development.

The final solution would need to be confirmed on individual development application and detail MUSIC modelling at development approval time. The measures would need to be able to be integrated into the overall public domain and recreation zones prior to discharge to the Parramatta River and achieving the nominated water quality objectives.



4.1.3 <u>Legal Point of Discharge</u>

The legal point of discharge is located in the north-east corner of the site via an existing 600mm RCP and associated 3.0m wide easement. The proposed development would utilise this connection and initial assessment shows there is sufficient capacity for the proposed development (incorporating the required on-site detention system) and 91 Bridge Road property (which also discharges via this point).



5 FLOODING

A *Flood Enquiry Application* has been made to Parramatta City Council. The information contained in the response includes council data on flood levels and drainage infrastructure contained in their database files. Reference to **Appendix E** can be made for the council *Flood Enquiry Application* letter (Council Reference: FL/28/2024 dated 28 February 2024).

The council application shows that there is a council drainage pipe which passes along the southern side of the site, past the legal point of discharge and east toward a local gully and north to the Upper Parramatta River.

The information shows the site to be clear of the 1 in 100-year flood level quoting a level of RL 20.1m AHD. This level is seen to be well below the minimum site level of RL 23.0m AHD.

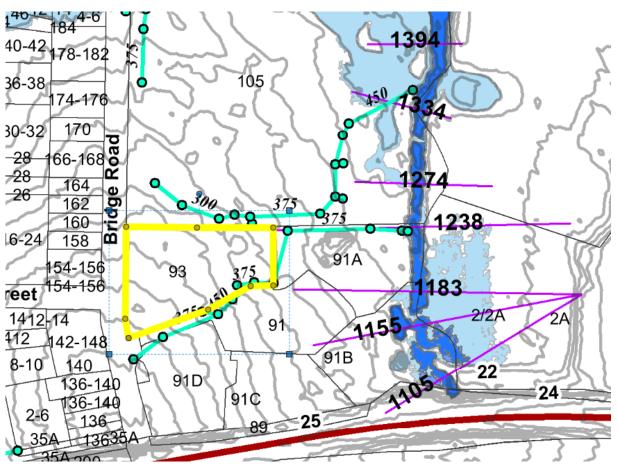


Figure 5.1. Parramatta Council Flood Extent Map

The flood enquiry information also shows the site will be clear of the PMF flood event extent.



The defined the Flood Planning Levels (FPL) for the site based on the 1 in 100 year ARI storm flood level plus 500mm freeboard, allowing for the development to be sited above the 1 in 100 year ARI flood level. The estimated FPL for the site is RL 20.6m AHD.

Based on the position of the property and local topography (near the top of the catchment and positioned along a ridge line), the site is not expected to be affected by any local overland flow paths.



6 CONCLUSION

This Site Conditions & Infrastructure Services Assessment Report has been prepared to support the proposal to adjust the FSR and height restrictions in relation to a proposed mixed use residential and commercial development at 93 Bridge Road, Westmead, NSW.

This would allow for future development of the land as residential, retail and recreational activities.

An overview of services and engineering considerations has been provided to assist in the stage one due diligence and planning application process for the proposed development of the site.

All major services can be seen to be located in the vicinity of the site. Generally all essential services are located directly adjacent to the site, and there is scope to either extend or upgrade these services to facilitate the proposed development. The servicing for the site would be completed at no cost to Government and would not impinge on the services of existing landowners.

Accordingly, based on the site conditions of the land and the availability of infrastructure services to the land, the development site and its extended land area provides an opportunity to make available additional development lands in a short timeframe.



7 **REFERENCES**

Development Engineering Design Guidelines – 2018

Managing Urban Stormwater: Harvesting and Reuse – 2006 (NSW DEC);

Managing Urban Stormwater: Source Control – 1998 (NSW EPA);

Managing Urban Stormwater: Treatment Techniques – 1997 (NSW EPA);

Managing Urban Stormwater: Soils & Construction – 2004 (LANDCOM);

Part 5, Development Control Plan (2023), City of Parramatta Council

On-site Stormwater Detention Handbook (Fourth Edition 2005), Upper Parramatta River Catchment Trust; and

Water Sensitive Urban Design - "Technical Guidelines for Western Sydney" by URS Australia Pty Ltd, May 2004

8 **APPENDICES**

Appendix A- Proposed Development Layout

Appendix B- Drawings by Costin Roe Consulting

Appendix C- Dial Before You Dig Information

Appendix D – Existing Site Survey

Appendix E – Council Flood Information



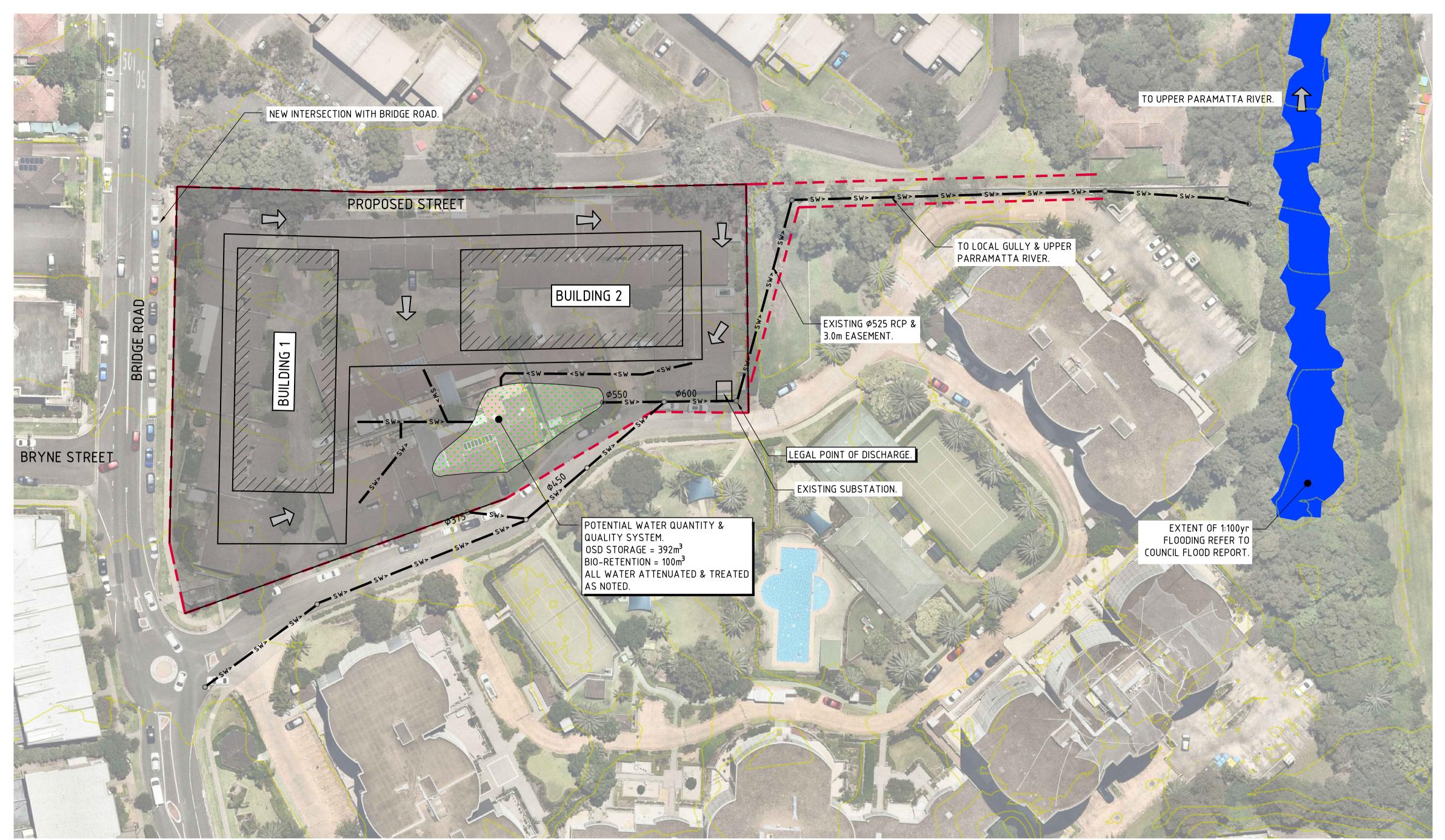
Appendix A Proposed Development Layout







Appendix B Drawings by Costin Roe Consulting





EXISTING CONTOUR (1m INTERVAL)

WATER QUALITY TARGETS

POLLUTANT	ANNUAL PERCENTAGE REDUCTION
GROSS POLLUTANTS TOTAL SUSPENDED SOLIDS TOTAL PHOSPHORUS TOTAL NITROGEN TOTAL HYDROCARBONS	90% 85% 65% 45% 90%

SITE WATER QUALITY TREATMENT TRAIN

THIS SITE WILL BE EFFECTIVE IN REDUCING THE POLLUTANT OF PHOSPHORUS, NITROGEN, SUSPENDED SOLIDS AND GROSS POLLUTANTS FROM STORMWATER RUNOFF PRIOR TO DISCHARGING FROM THE SITE

STORMWATER QUANTITY NOTE

THIS SITE WILL PROVIDE EFFECTIVE STORMWATER, THROUGH THE PROVISION OF AN ON-SITE DETENTION (OSD) SYSTEM, ATTENUATION FOR STORMS OF 1 IN 2 YEAR ARI THROUGH TO 1 IN 100 YEAR ARI.

THE OSD SYSTEM WILL BE ADEQUATELY SIZED TO MEET THE REQUIREMENTS OF PARRAMATTA COUNCIL & THE UPPER PARRAMATTA RIVER CATCHMENT TRUST ON-SITE DETENTION HANDBOOK.



FOR INFORMATION

SCALE 1:500 AT A1 SIZE SHEET

ARCHITECT ISSUED FOR INFORMATION 09.02.24 A DATE ISSUE

PROPOSED UNIT TRUST 93 BRIDGE ROAD, WESTMEAD NSW, 2145

DESIGNED DRAWN DATE CHECKED SIZE SCALE CAD REF:
CC CC FEB'24 XC A1 AS SHOWN C013829.01-SK01

CLIENT

Costin Roe Consulting Pty Ltd.
ABN 50 003 696 446

f: +61 2 9241 3731

PO Box N419 Sydney NSW 1220 Level 4, 8 Windmill Street, Millers Point NSW 2000

e: mail@costinroe.com.au w: costinroe.com.au

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CONSULTING

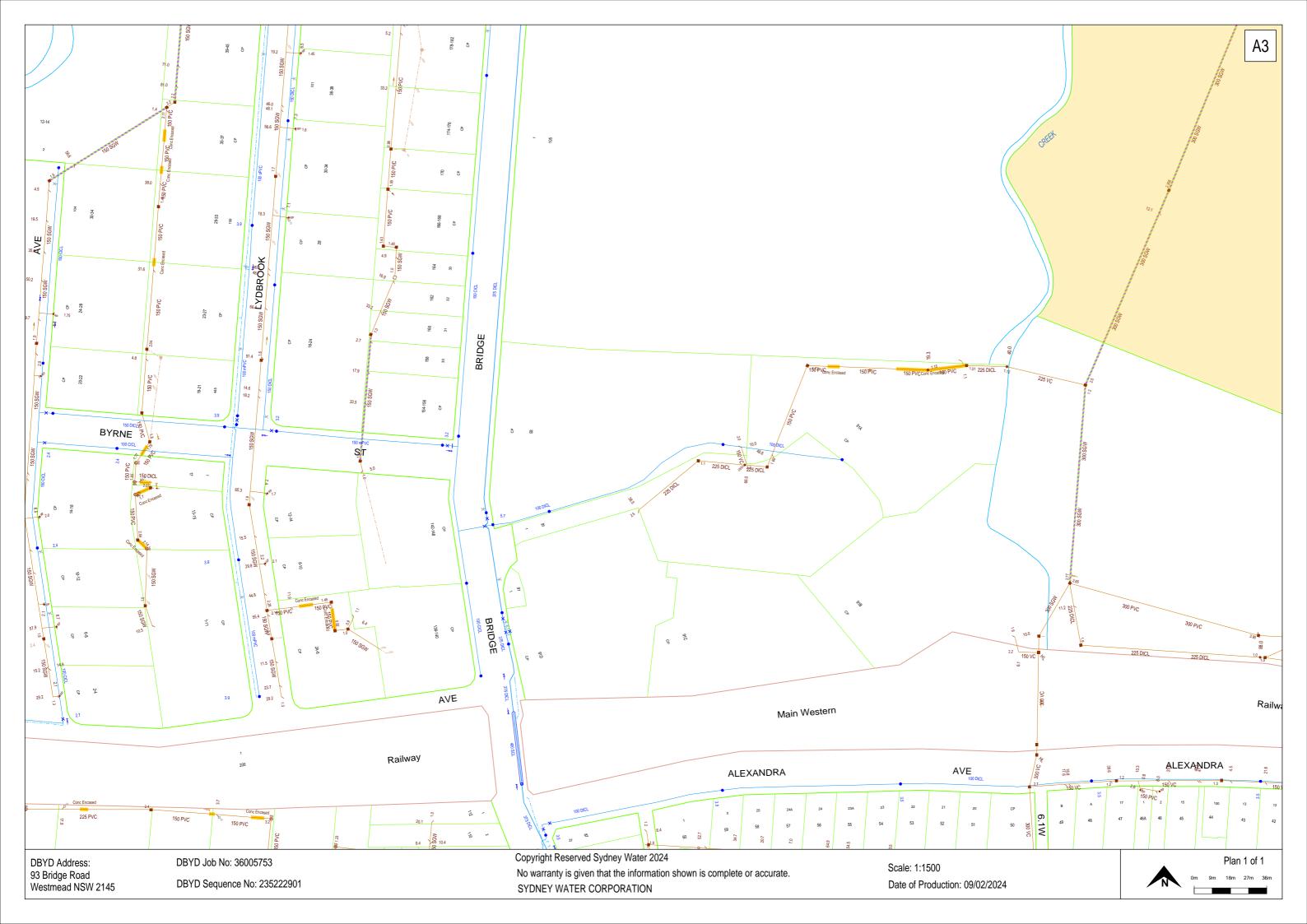
CIVIL & STRUCTURAL **ENGINEERS**

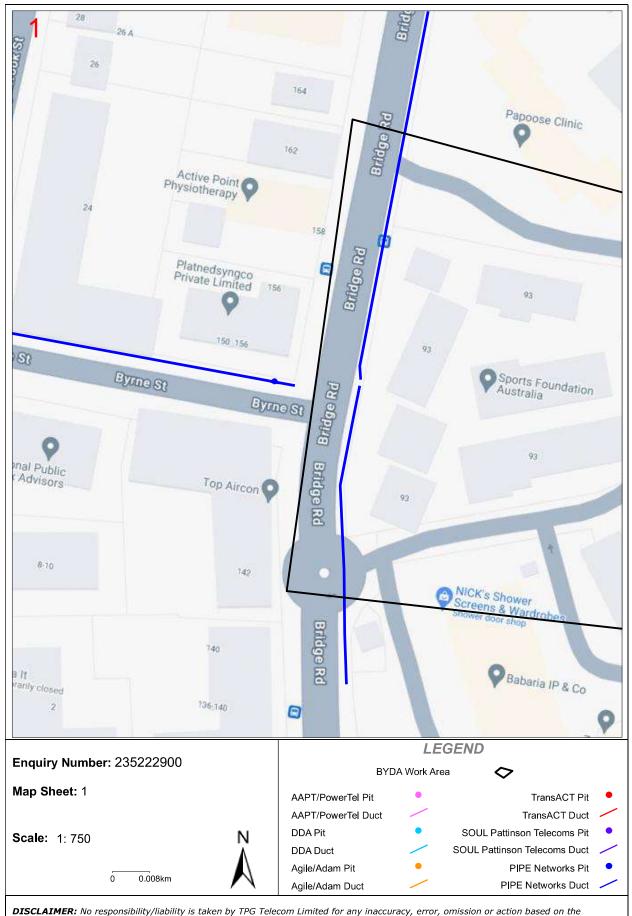
INDICATIVE STORMWATER CONCEPT PLAN

DRAWING No CO13829.01-SK01

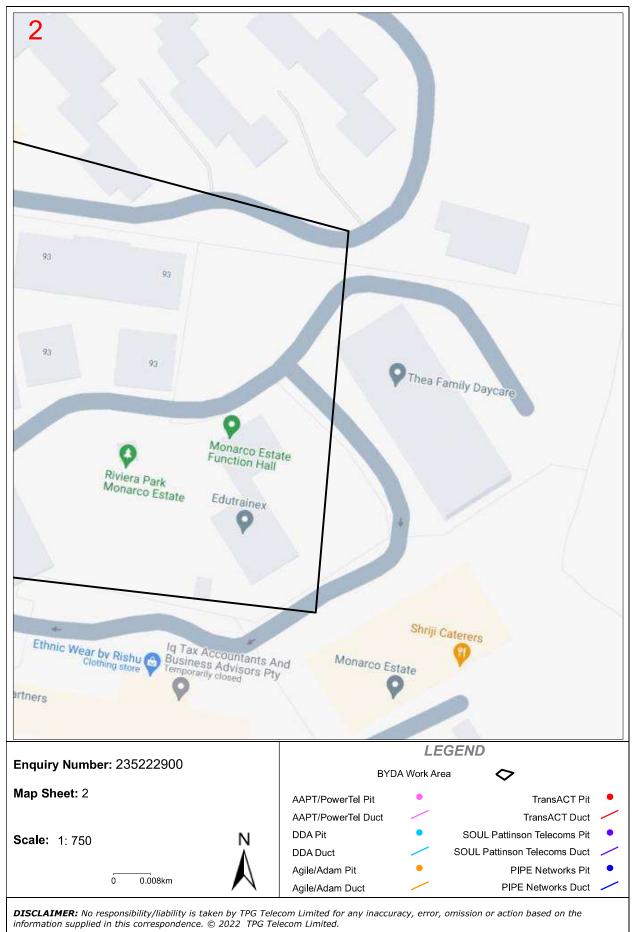


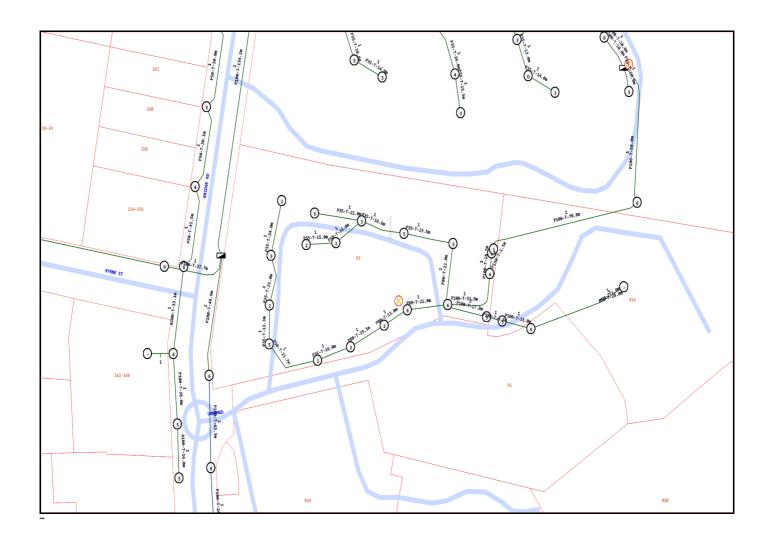
Appendix CDial Before You Dig Information





DISCLAIMER: No responsibility/liability is taken by TPG Telecom Limited for any inaccuracy, error, omission or action based on the information supplied in this correspondence. © 2022 TPG Telecom Limited.

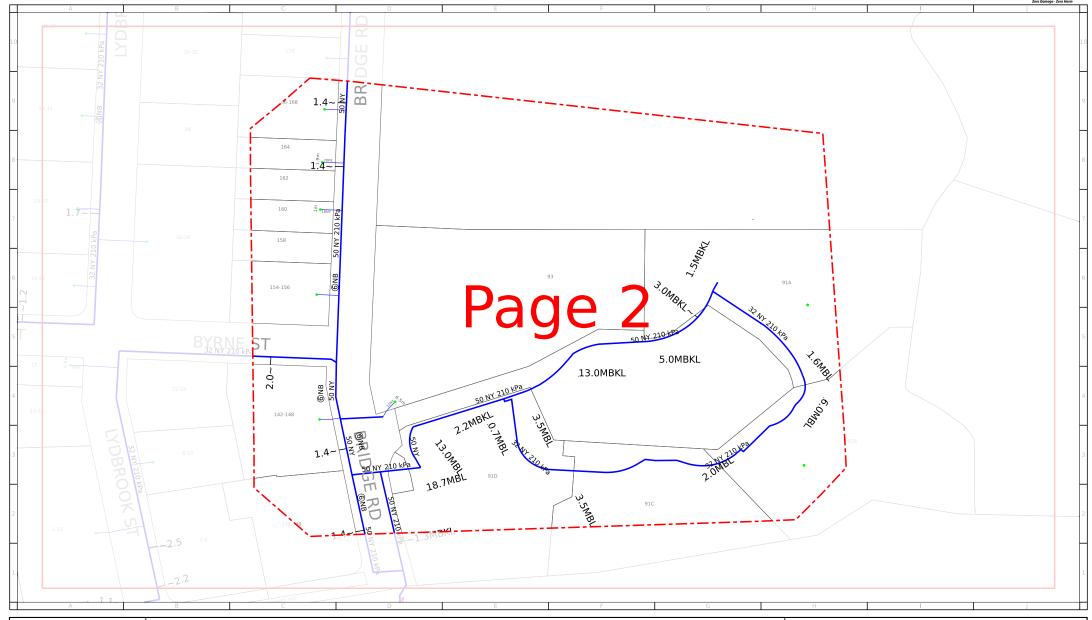




Emergency Contacts

You must immediately report any damage to the ${\bf nbn}^{\,{\rm m}}$ network that you are/become aware of. Notification may be by telephone - 1800 626 329.







For legend details, please refer to the Coversheet attachment provided as part of this BYDA response.

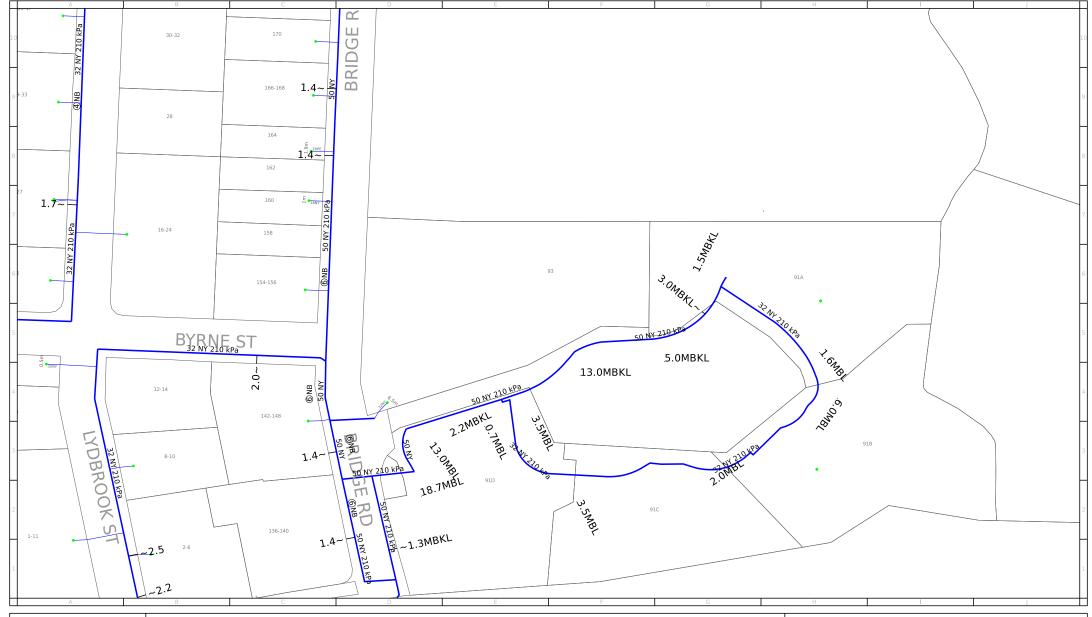


Issue Date: 09/02/2024
BYDA Seq No: 235222898
BYDA Job No: 36005753

Overview Page:

Scale:1:2101







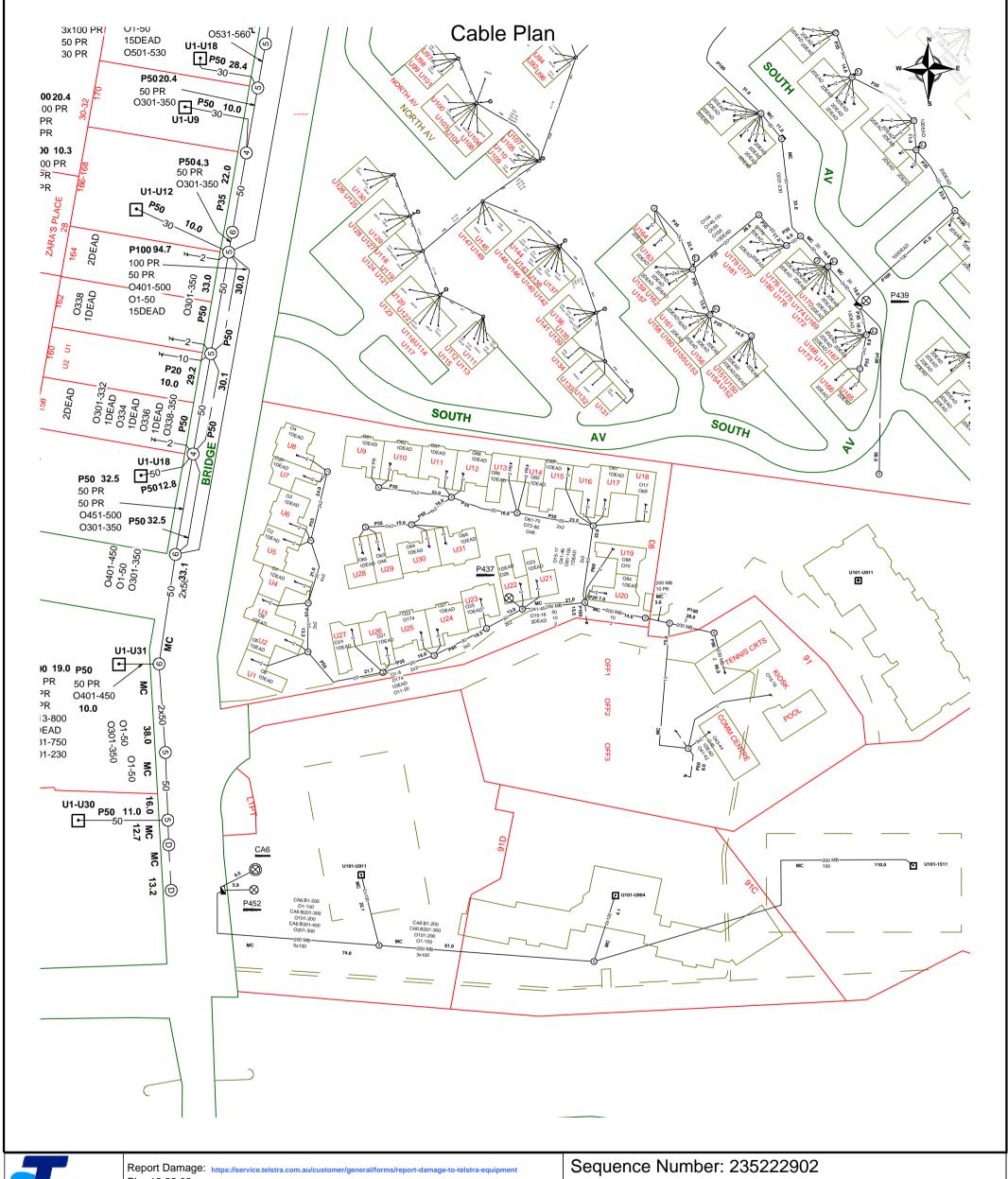
For legend details, please refer to the Coversheet attachment provided as part of this BYDA response.



Issue Date: 09/02/2024 BYDA Seq No: 235222898 BYDA Job No: 36005753

Scale:1:2000

0m 10m 20m 30m 40m 50m 60m 70m80m





Ph - 13 22 03

Email - Telstra.Plans@team.telstra.com

Planned Services - ph 1800 653 935 (AEST bus hrs only) General Enquiries

TELSTRA LIMITED A.C.N. 086 174 781

Generated On 09/02/2024 14:20:36

CAUTION: Fibre optic and/ or major network present in plot area. Please read the Duty of Care and contact Telstra Plan Services should you require any assistance.

The above plan must be viewed in conjunction with the Mains Cable Plan on the following page

WARNING

Telstra plans and location information conform to Quality Level "D" of the Australian Standard AS 5488-Classification of Subsurface Utility Information.

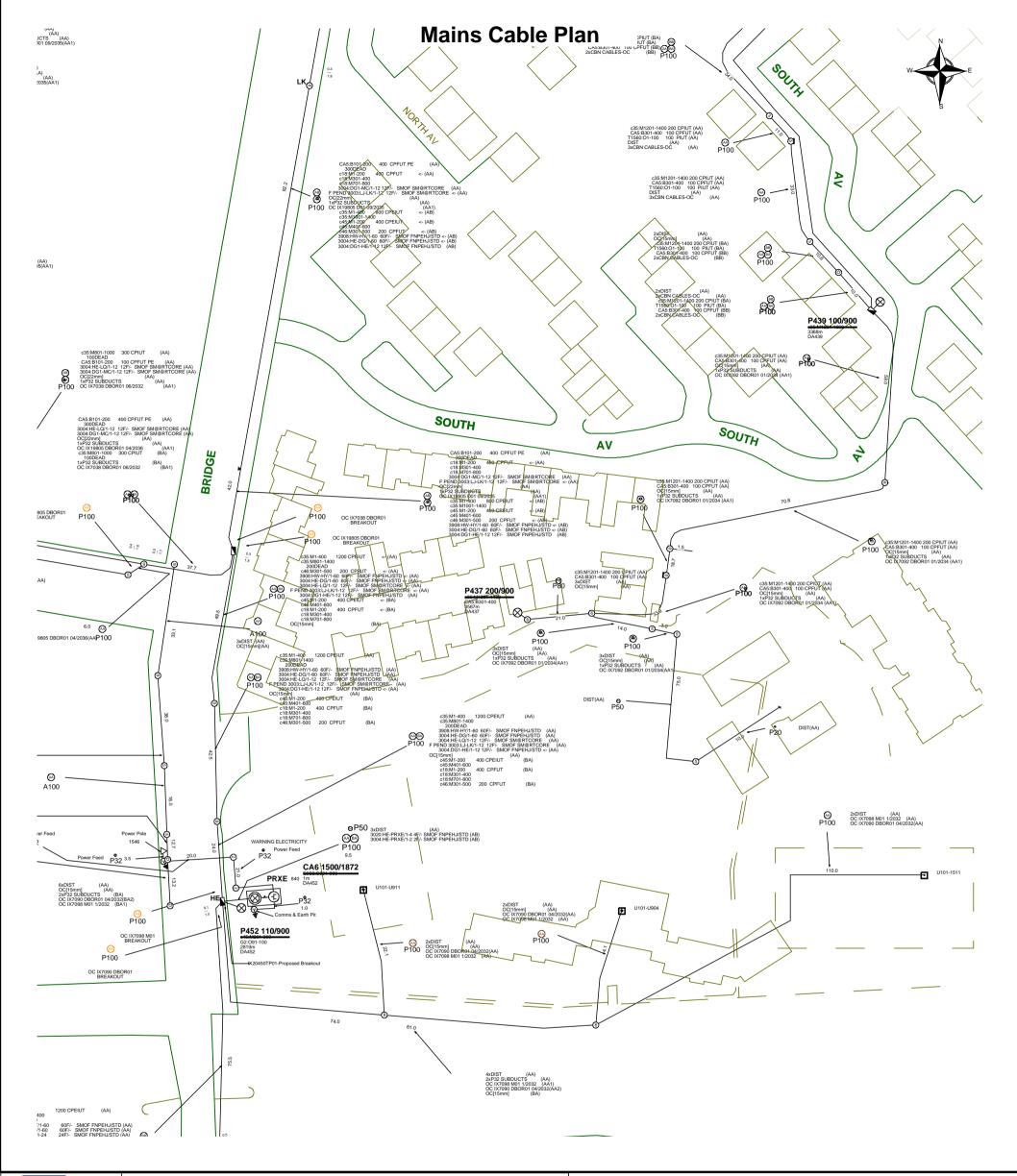
 $As \ such, Telstra \ supplied \ location \ information \ is \ indicative \ only. Spatial \ accuracy \ is \ not \ applicable \ to \ Quality \ Level \ D.$

Refer to AS 5488 for further details. The exact position of Telstra assets can only be validated by physically exposing it.

Telstra does not warrant or hold out that its plans are accurate and accepts no responsibility for any inaccuracy. Further on site investigation is required to validate the exact location of Telstra plant prior to commencing construction work.

A Certified Locating Organisation is an essential part of the process to validate the exact location of Telstra assets and to ensure the asset is protected during construction works.

See the Steps-Telstra Duty of Care that was provided in the email response.



 $Report\ Damage:\ {}_{https://service.telstra.com.au/customer/general/forms/report-damage-to-telstra-equipment of the company of the company$

Ph - 13 22 03

Email - Telstra.Plans@team.telstra.com

Planned Services - ph 1800 653 935 (AEST bus hrs only) General Enquiries

TELSTRA LIMITED A.C.N. 086 174 781

Generated On 09/02/2024 14:20:38

Sequence Number: 235222902

CAUTION: Fibre optic and/ or major network present in plot area. Please read the Duty of Care and contact Telstra Plan Services should you require any assistance.

WARNING

Telstra plans and location information conform to Quality Level "D" of the Australian Standard AS 5488-Classification of Subsurface Utility Information.

As such, Telstra supplied location information is indicative only. Spatial accuracy is not applicable to Quality Level D.

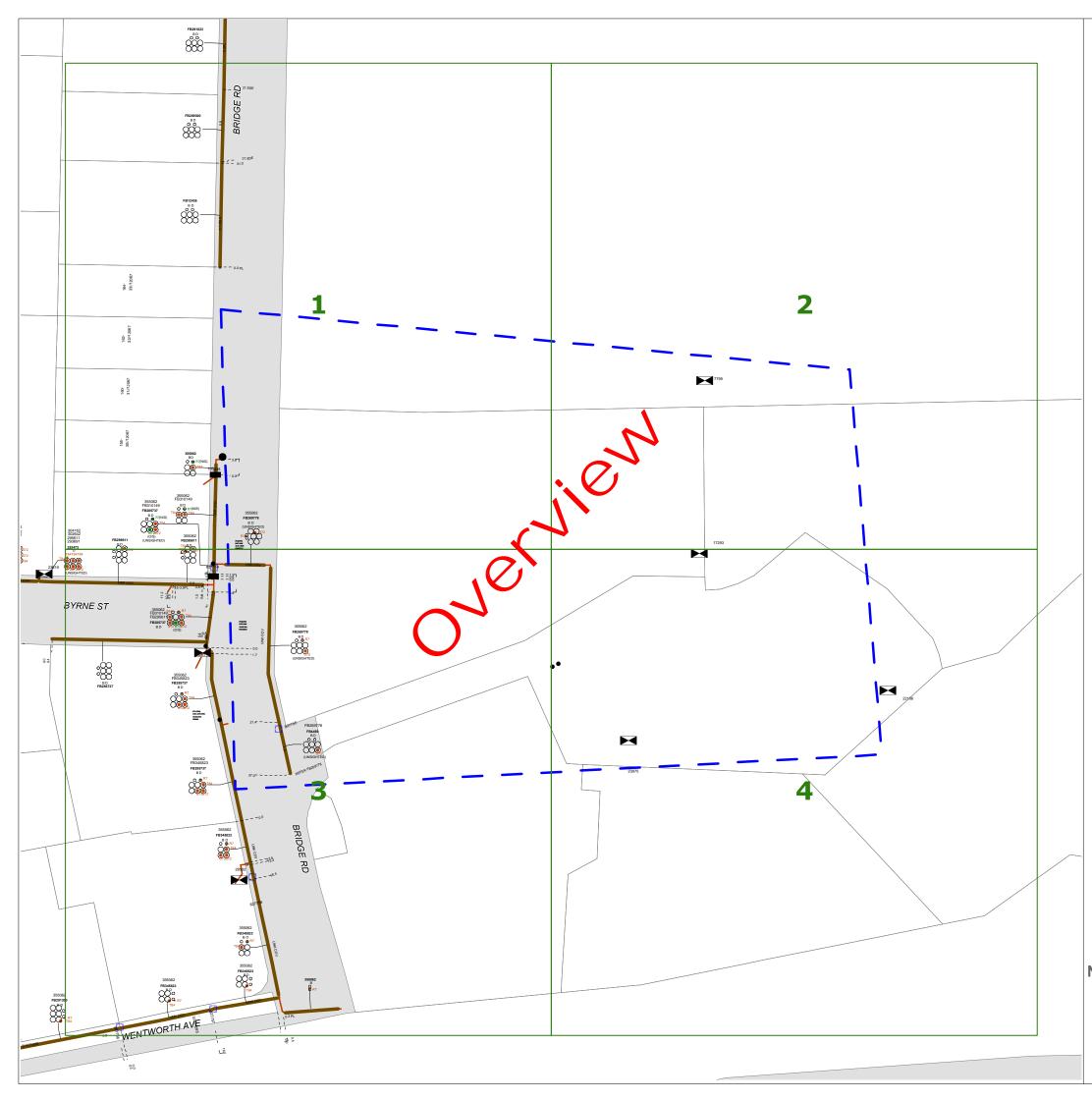
Refer to AS 5488 for further details. The exact position of Telstra assets can only be validated by physically exposing it.

Telstra does not warrant or hold out that its plans are accurate and accepts no responsibility for any inaccuracy.

Further on site investigation is required to validate the exact location of Telstra plant prior to commencing construction work.

A Certified Locating Organisation is an essential part of the process to validate the exact location of Telstra assets and to ensure the asset is protected during construction works.

See the Steps- Telstra Duty of Care that was provided in the email response.





WARNING

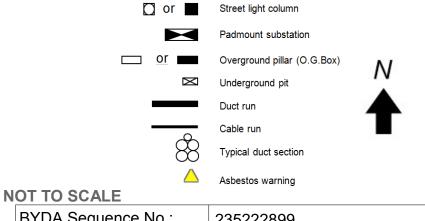
- All electrical apparatus shall be regarded as live until proved de-energised.
 Contact with live electrical apparatus will cause severe injury or death.
- Underground assets may be congested at the approach to bridges and other structures. Typical asset depths and alignment may vary substantially, rising and falling sharply and at much shallower depths than elsewhere as they are channelled into shared allocated spaces on bridges and other structures. Additional precautions and underground asset location methods will be required in proximity to bridges and other structures.
- In accordance with the *Electricity Supply Act 1995*, you are obliged to report any damage to Endeavour Energy Assets immediately by calling **131 003**.
- The customer must obtain a new set of plans from Endeavour Energy if work has not been started or completed within twenty (20) working days of the original plan issue date.
- The customer must contact Endeavour Energy if any of the plans provided have blank pages, as some underground asset information may be incomplete.
- Endeavour Energy underground earth grids may exist and their location **may not** be shown on plans. Persons excavating are expected to exercise all due care, especially in the vicinity of padmount substations, pole mounted substations, pole mounted switches, transmission poles and towers.
- Endeavour Energy plans **do not** show any underground customer service mains or information relating to service mains within private property.
- Asbestos or asbestos-containing material may be present on or near Endeavour Energy's underground assets.
- Organo-Chloride Pesticides (OCP) may be present in some sub-transmission trenches.
- All plans must be made available at the worksite where excavation is to be undertaken in either printed or electronic format. If the plans are in an electronic format, they must be in a format visible on a screen size 10 inches or greater. Plans must be reviewed and understood by the crew on site prior to commencing excavation.
- Non-destructive water excavation must be operated at or below 2000PSI. Any operation exceeding 2000PSI must be classed and treated as a destructive excavation practice

INFORMATION PROVIDED BY ENDEAVOUR ENERGY

- Any plans provided pursuant to this service are intended to show the approximate location of underground assets relative to road boundaries, property fences and other structures at the time of installation.
- Depth of underground assets may vary significantly from information provided on plans as a result of changes to road, footpath or surface levels subsequent to installation.
- Such plans have been prepared solely for use by Endeavour Energy staff for design, construction and maintenance purposes.
- All enquiry details and results are kept in a register.

DISCLAIMER

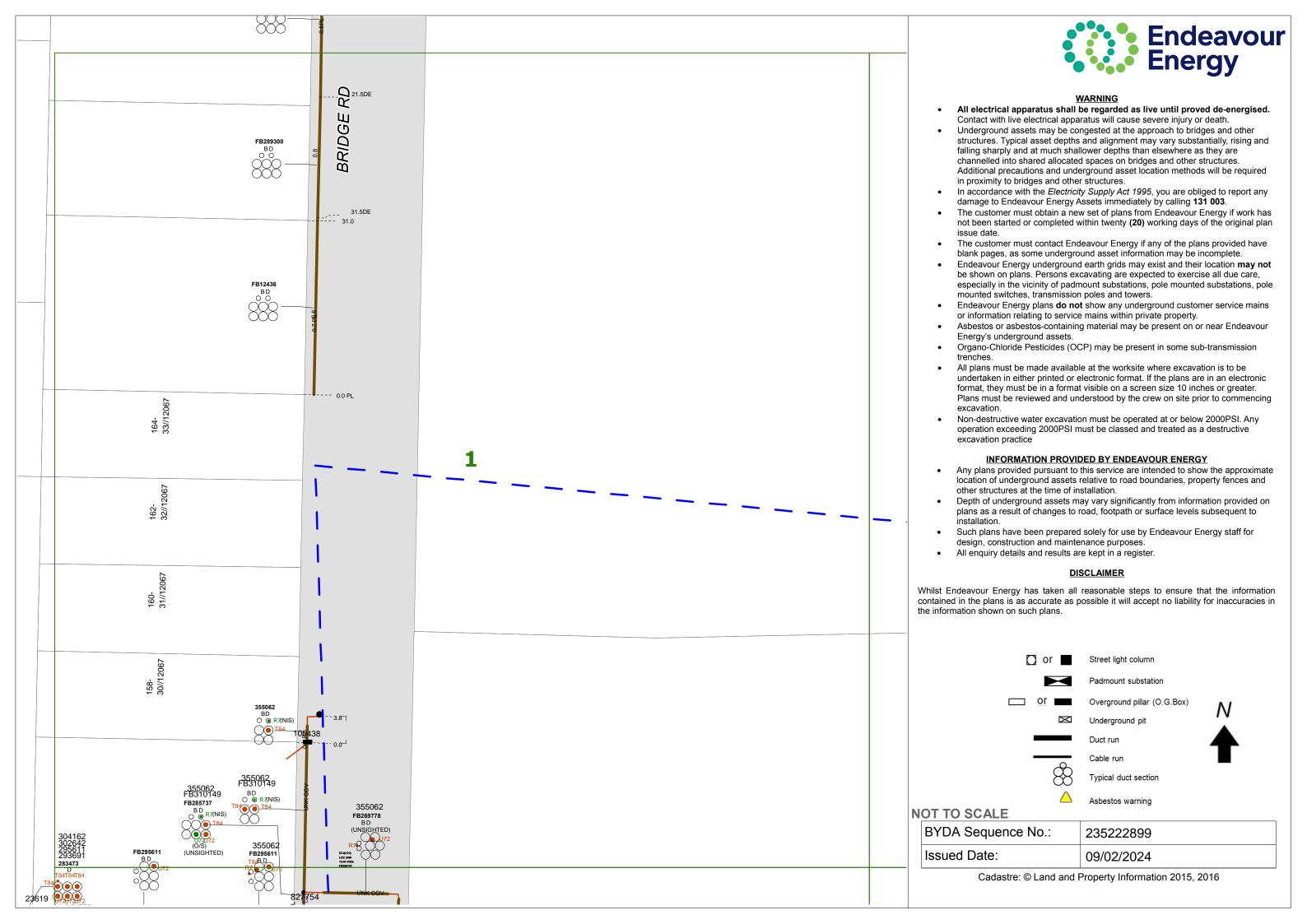
Whilst Endeavour Energy has taken all reasonable steps to ensure that the information contained in the plans is as accurate as possible it will accept no liability for inaccuracies in the information shown on such plans.

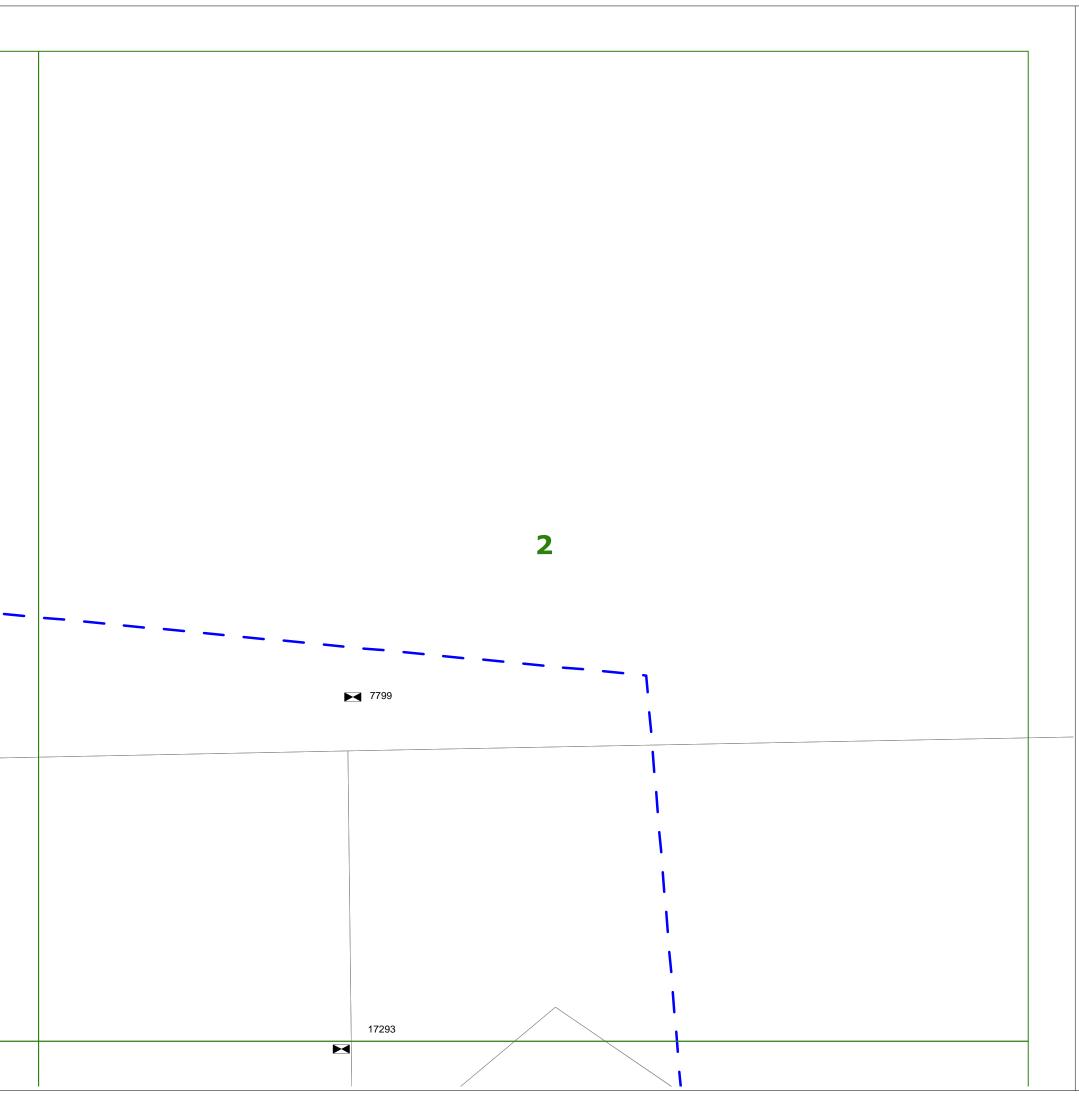


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Issued Date: 09/02/2024

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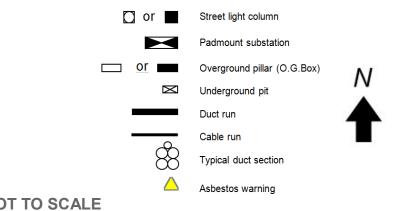
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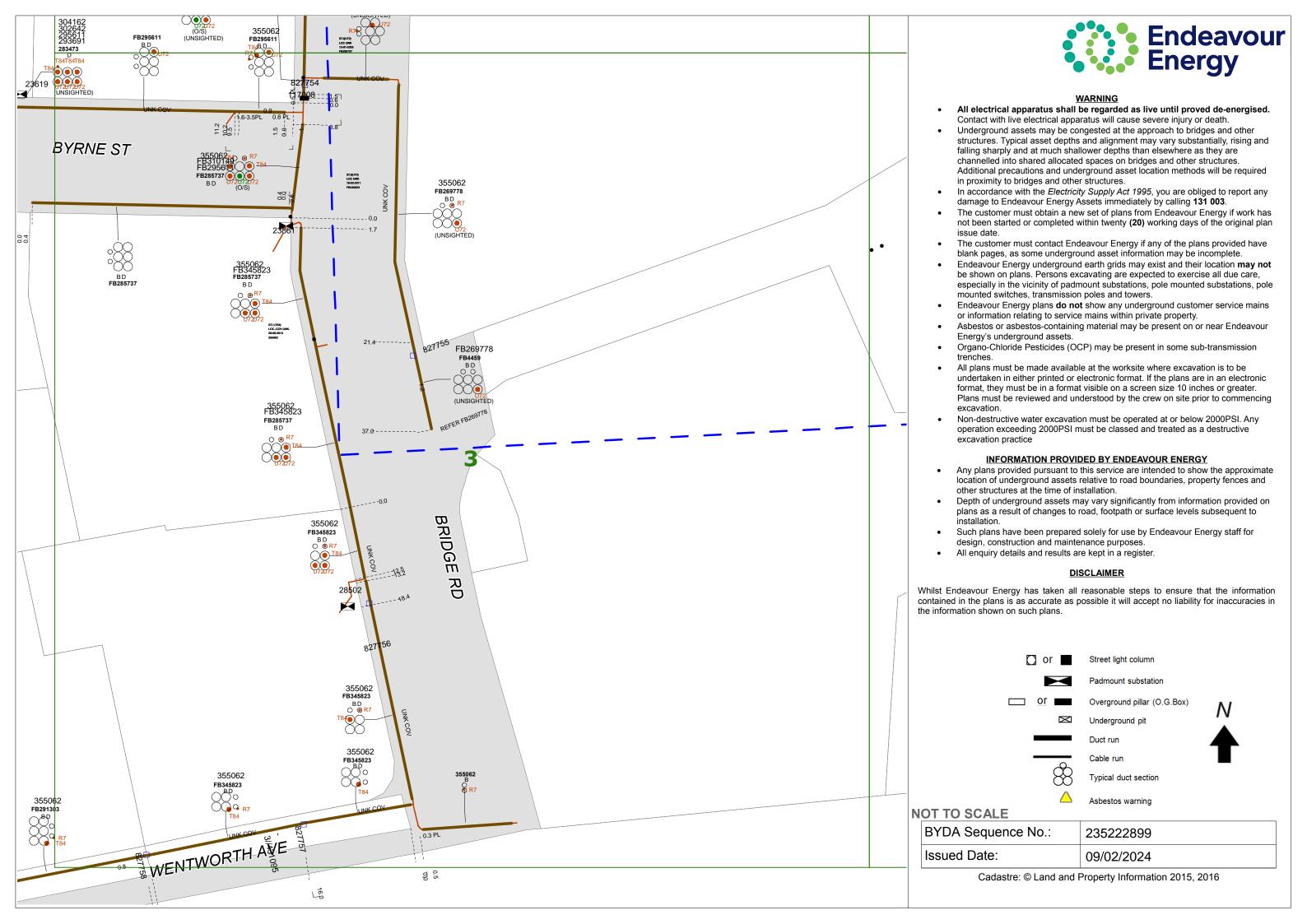
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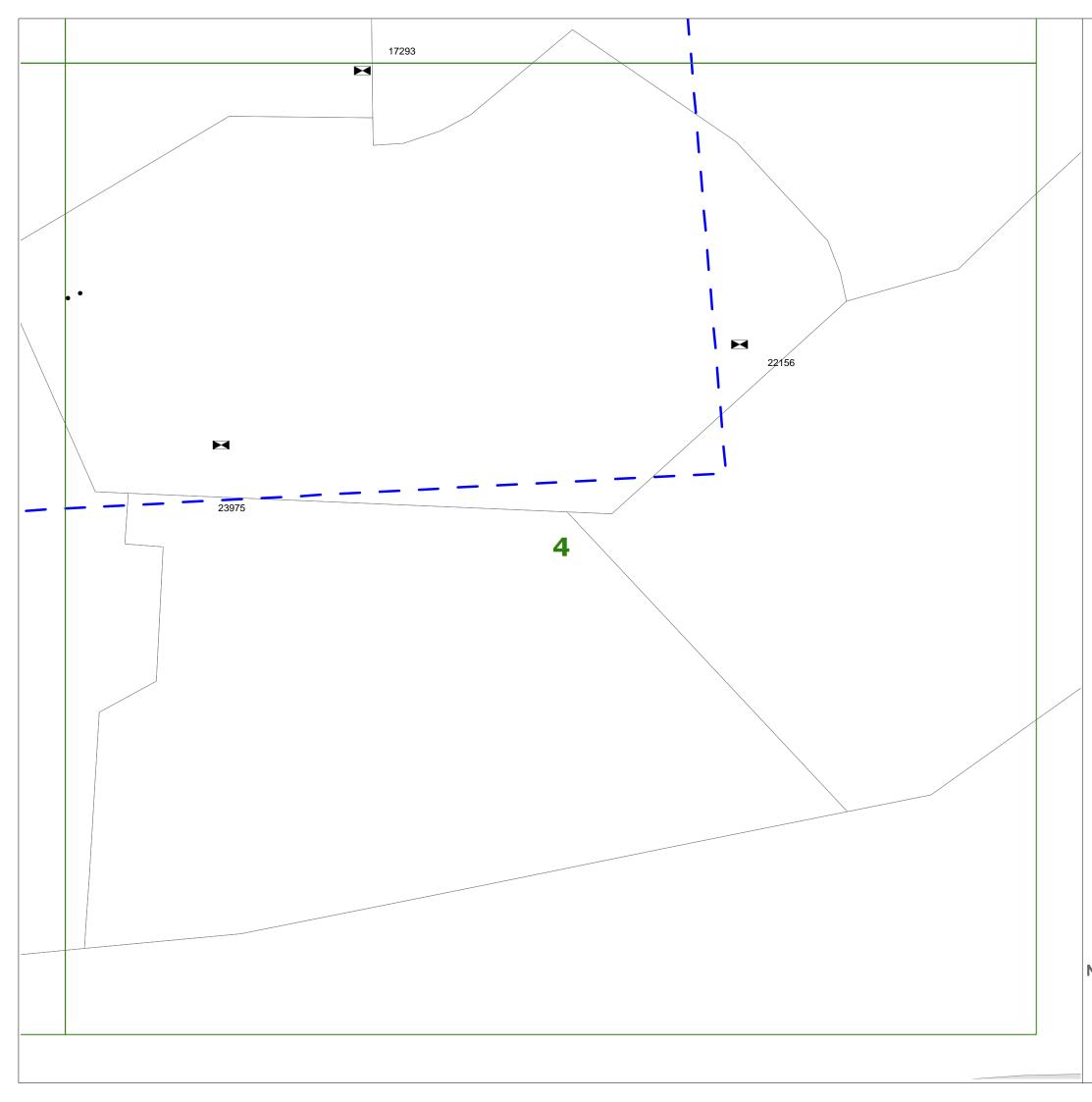


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BYDA Sequence No.:	235222899
Issued Date:	09/02/2024

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WARNING

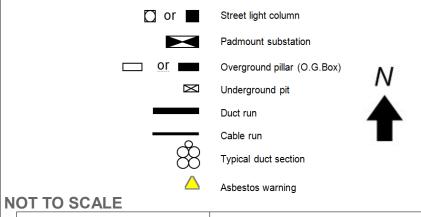
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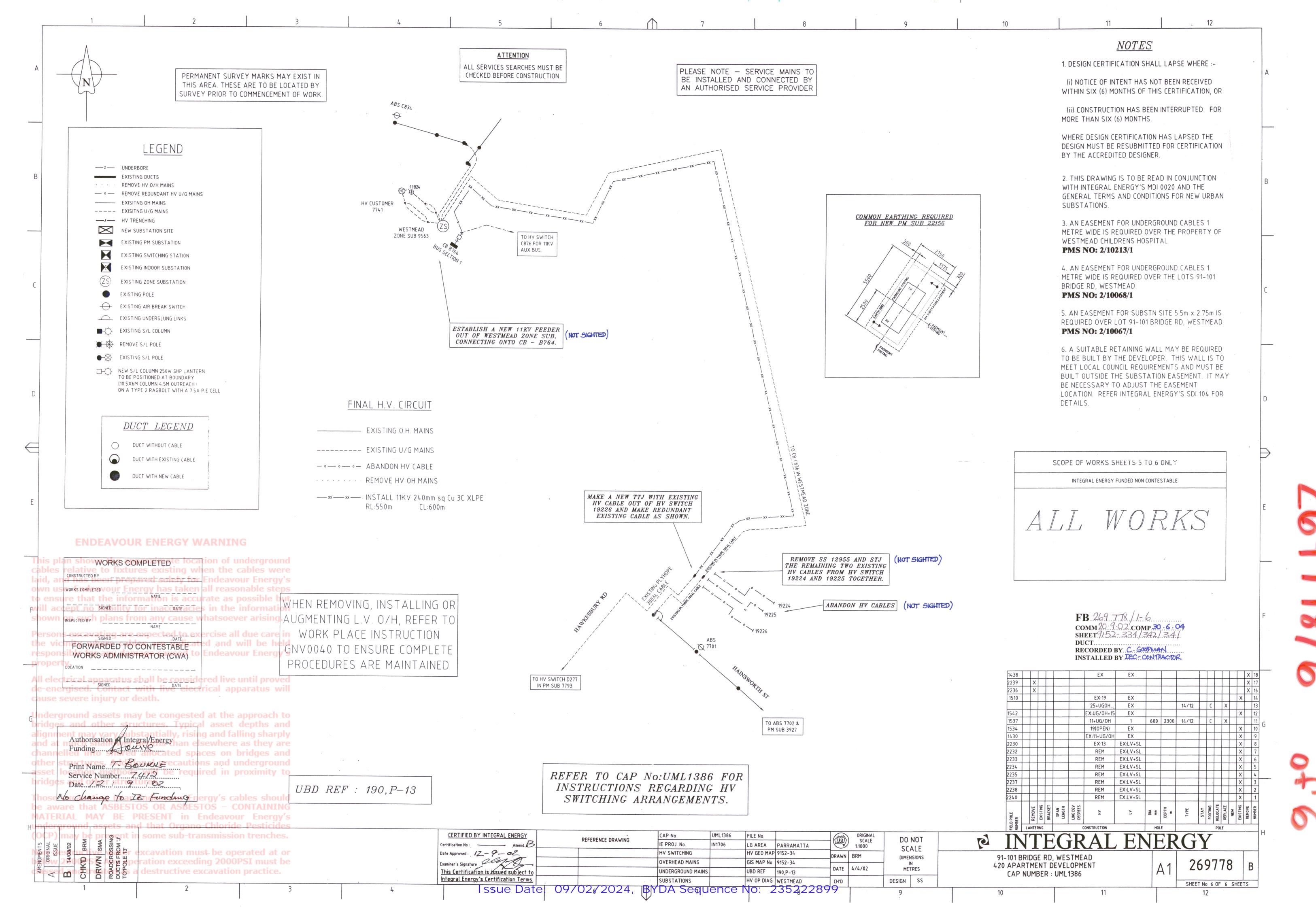


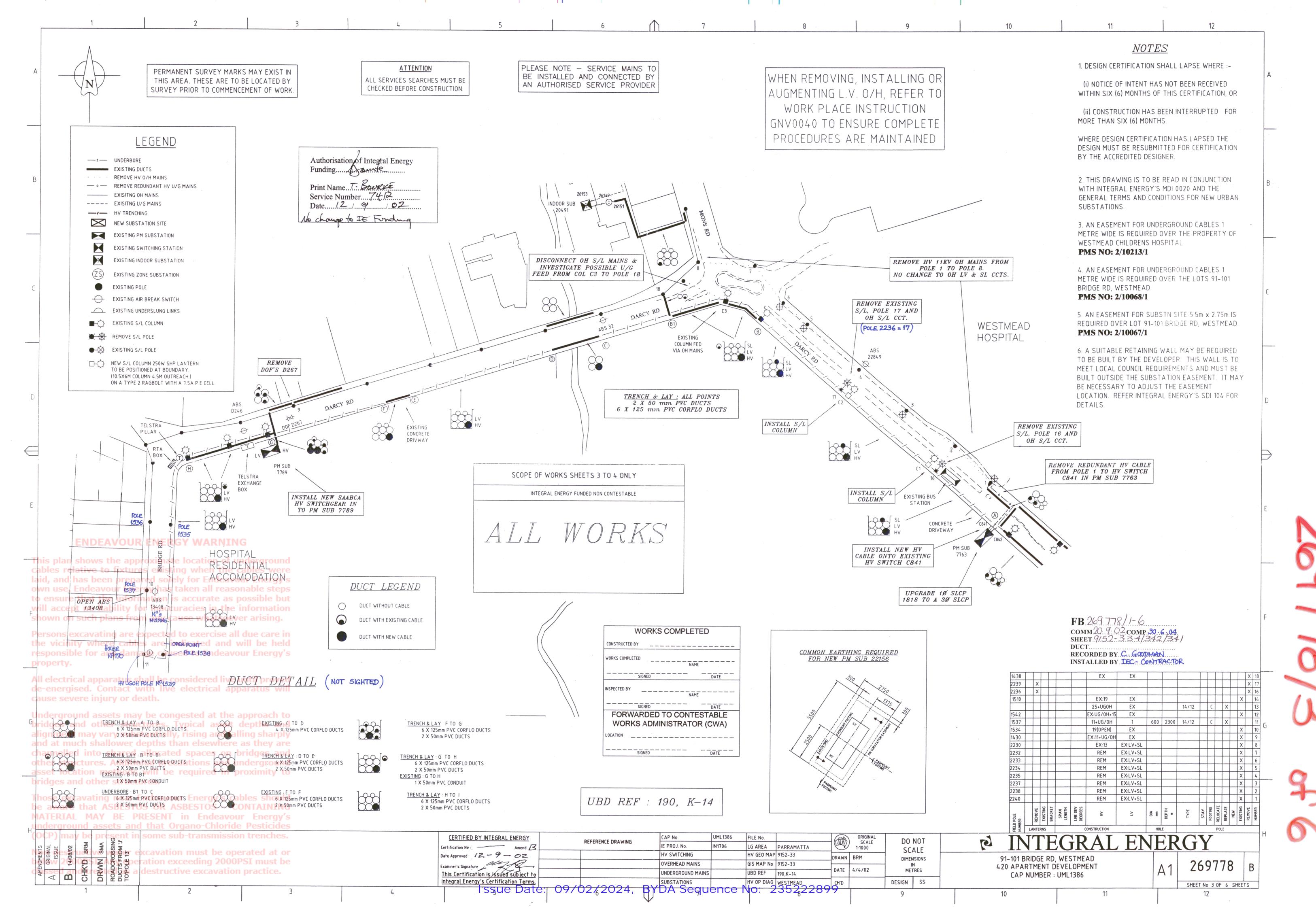
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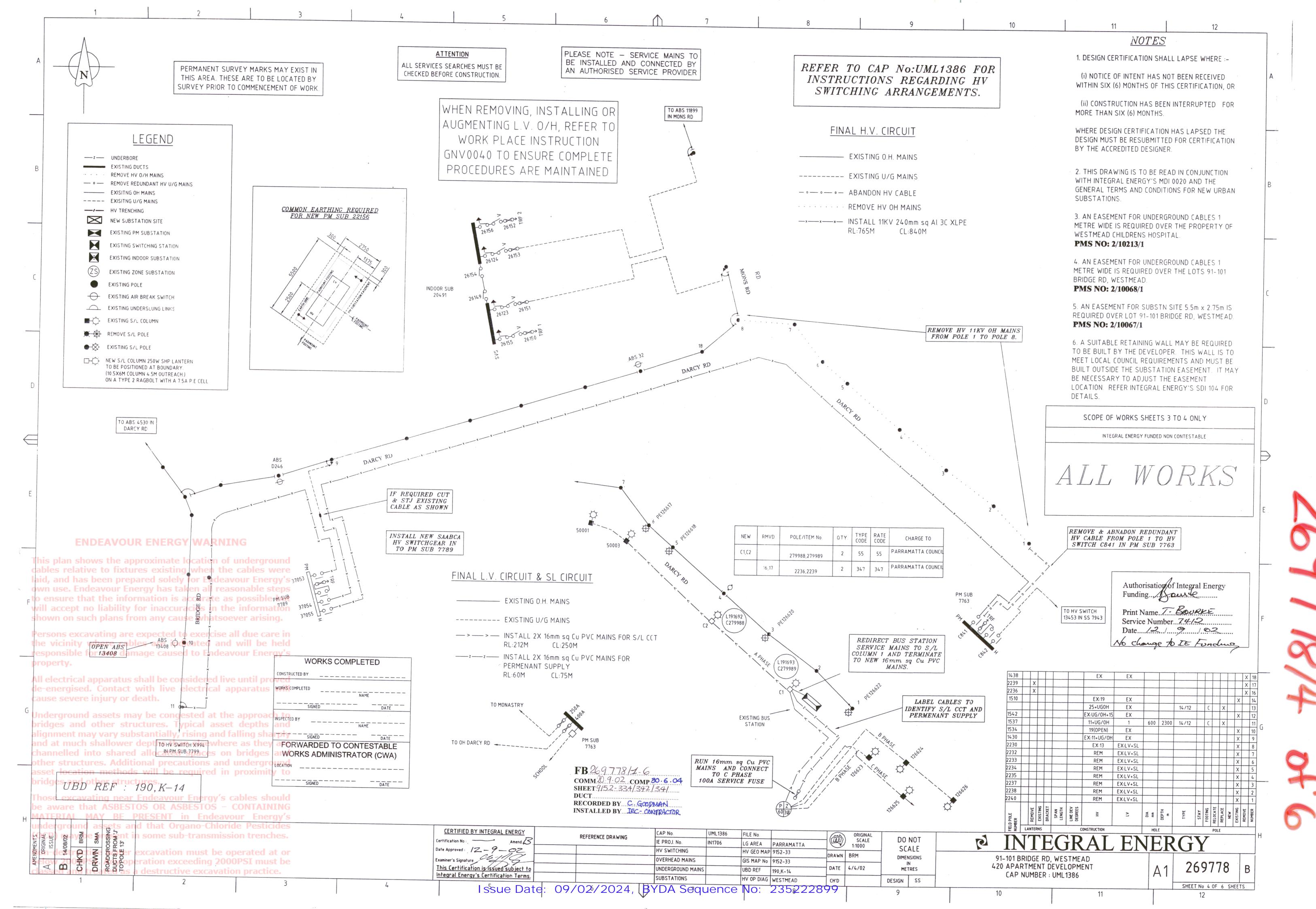
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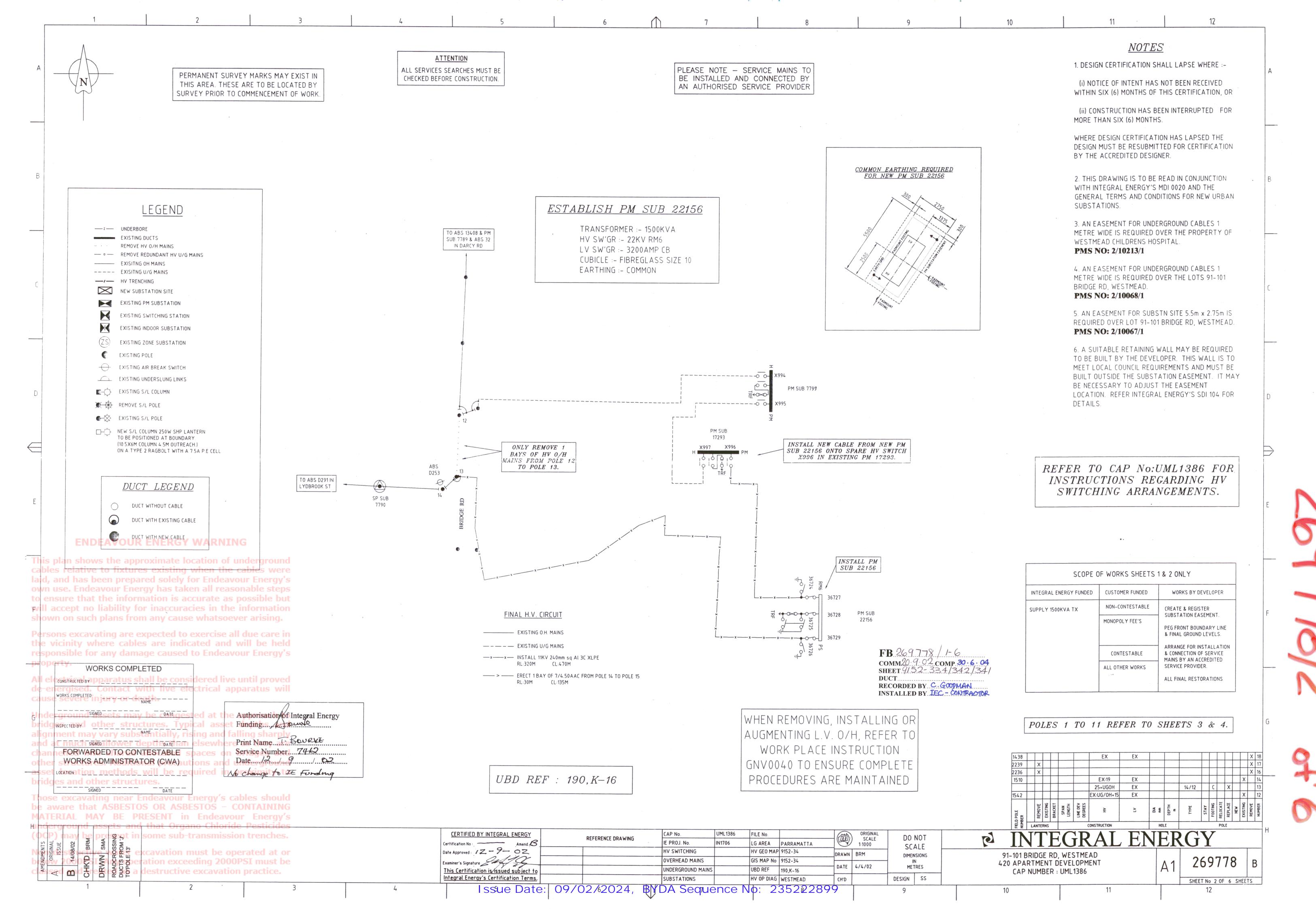
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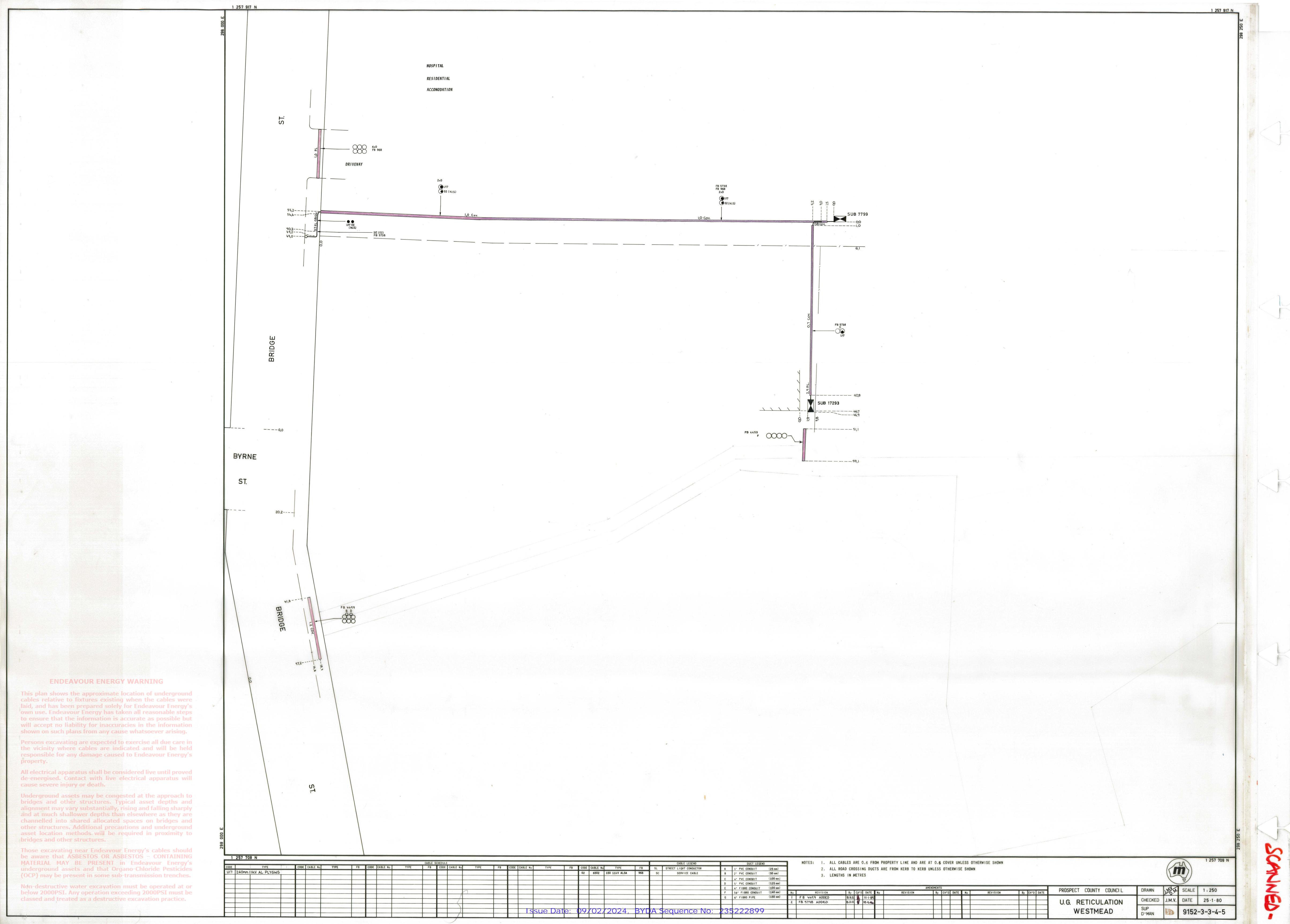
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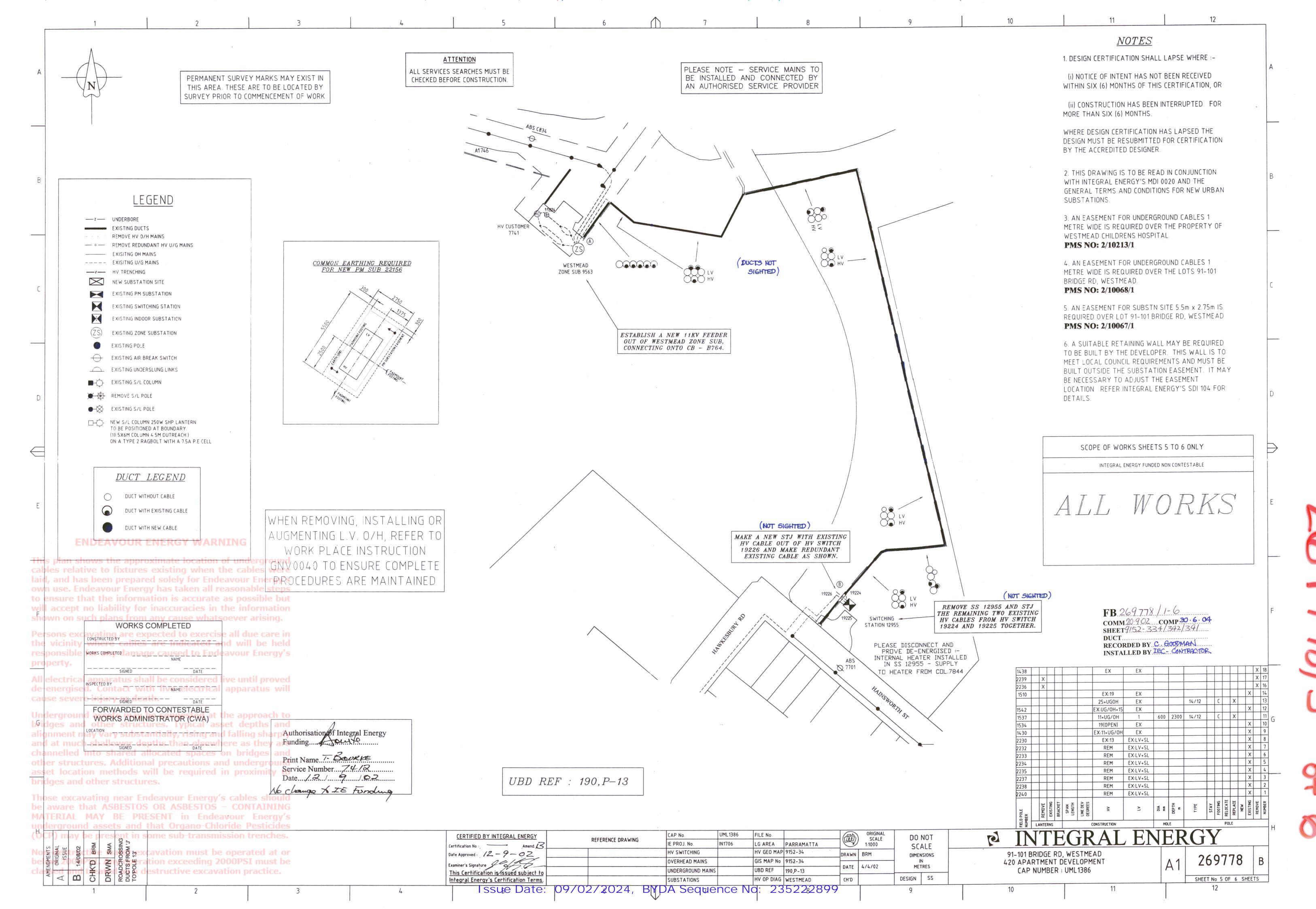


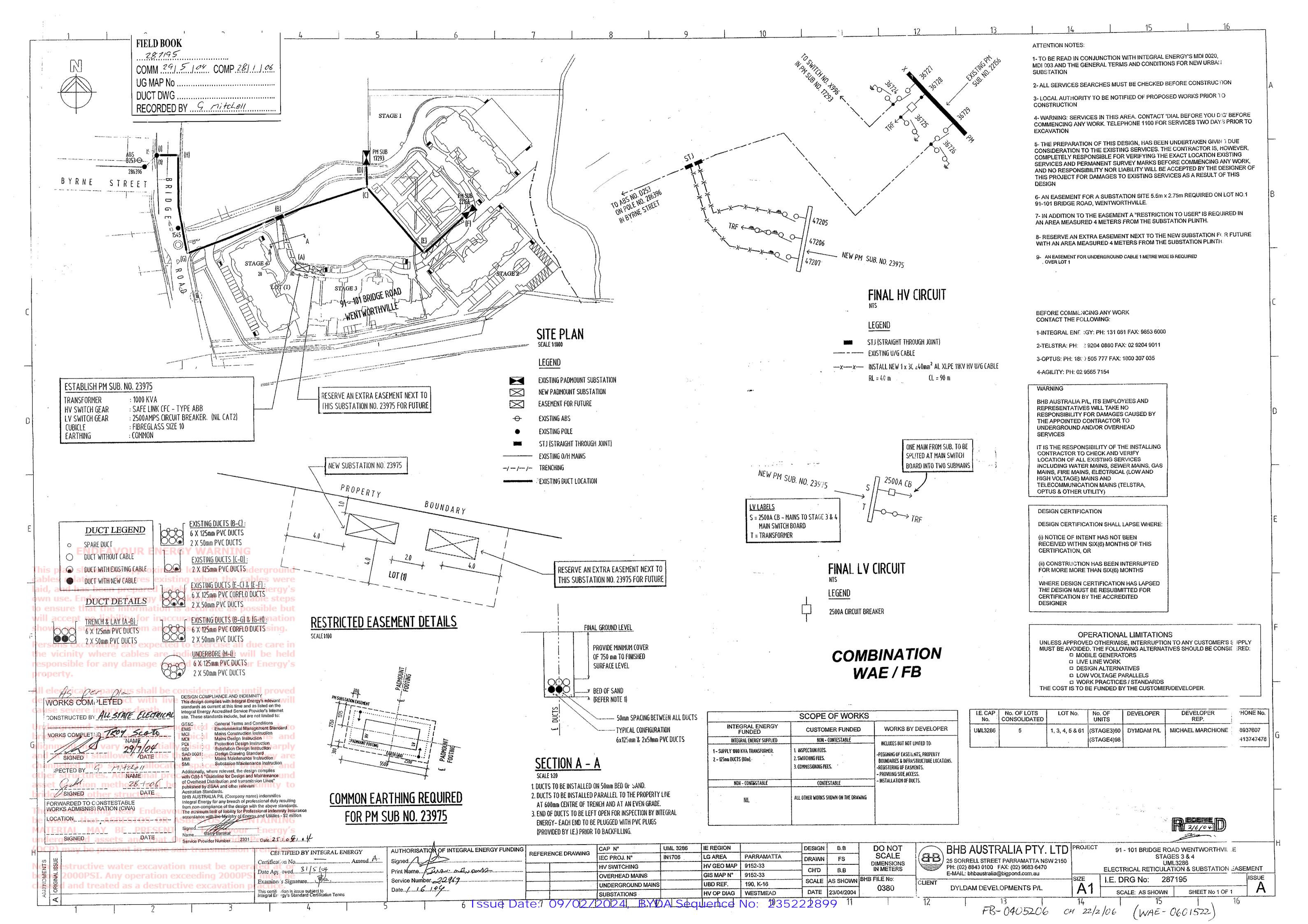


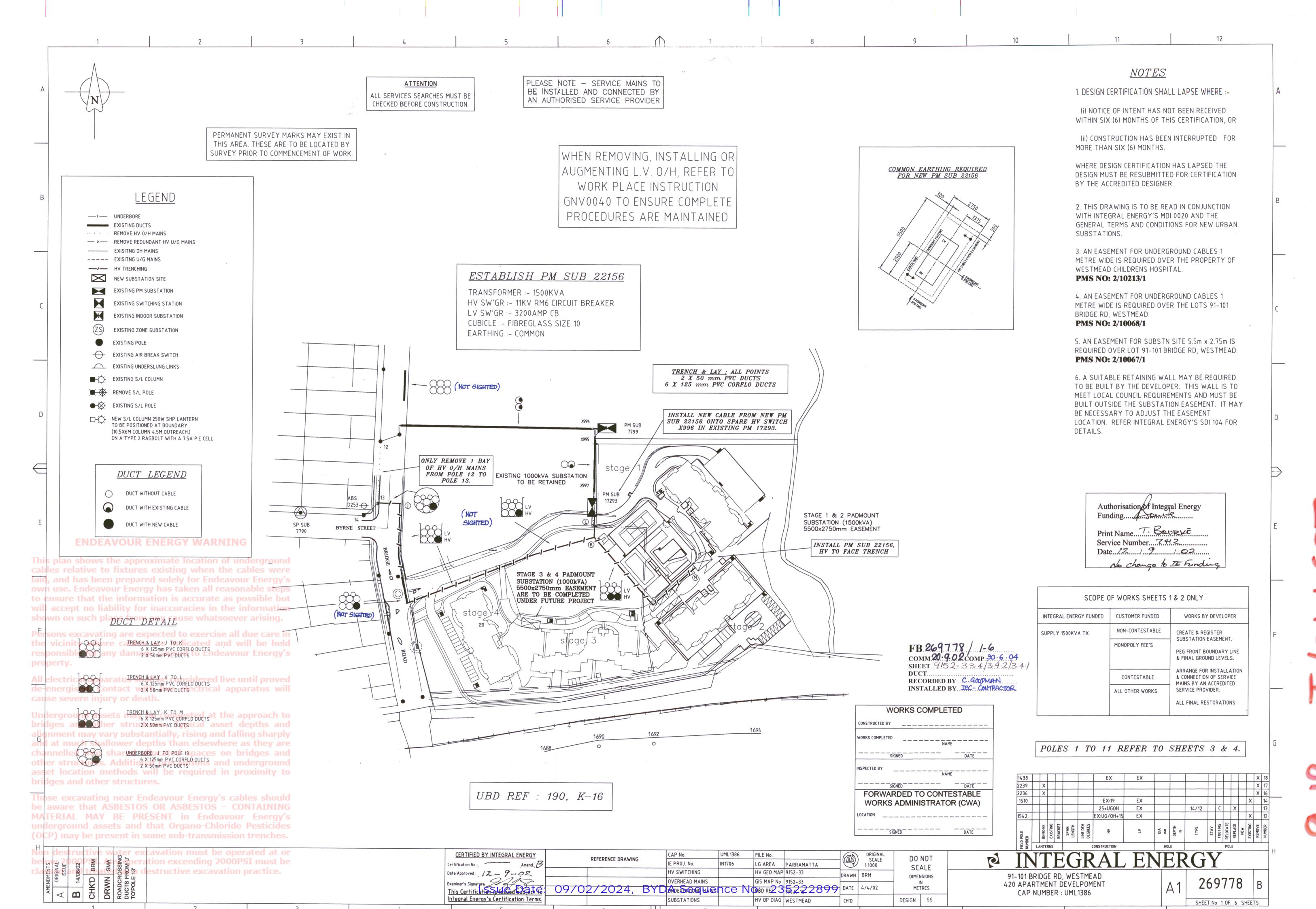






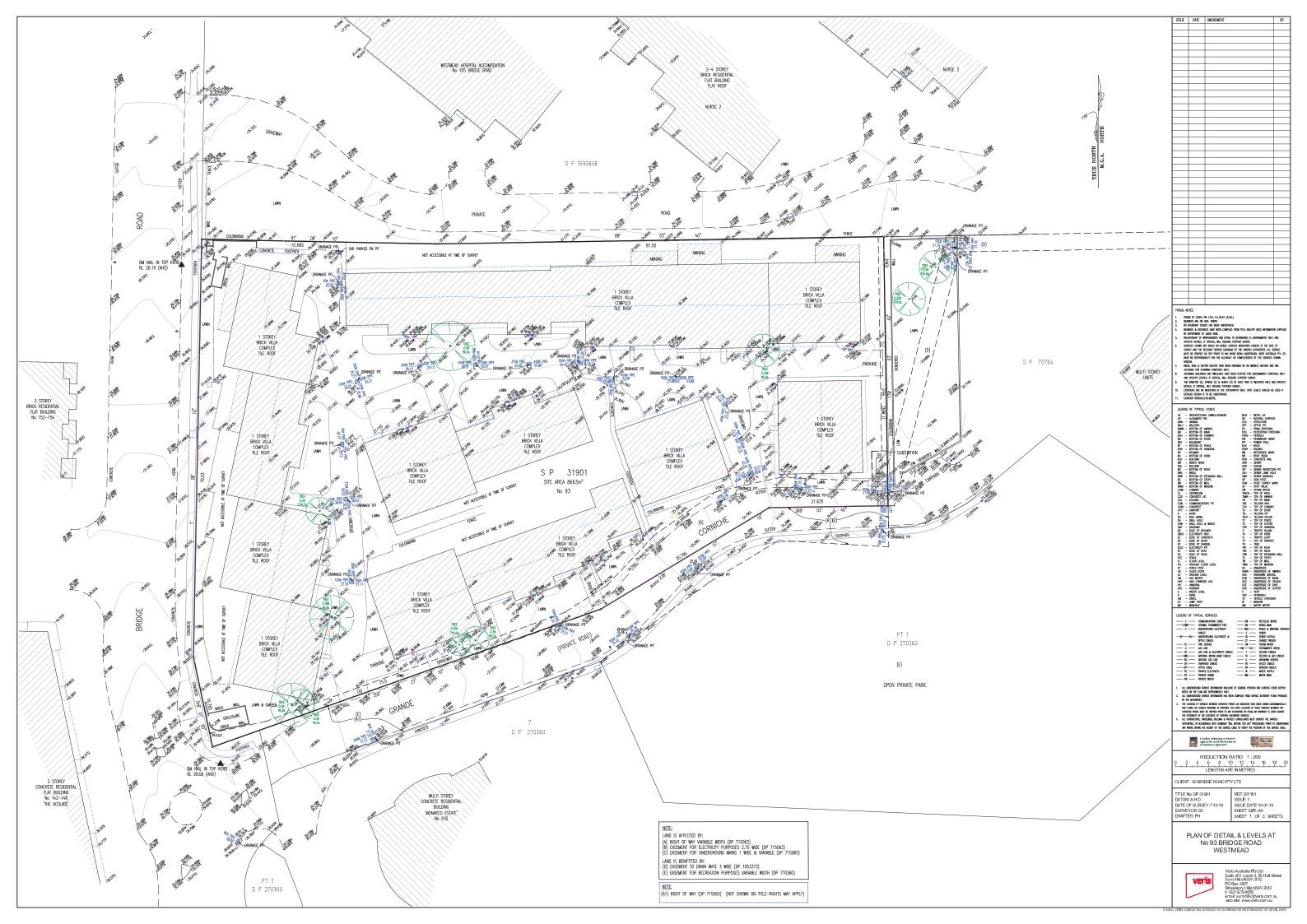


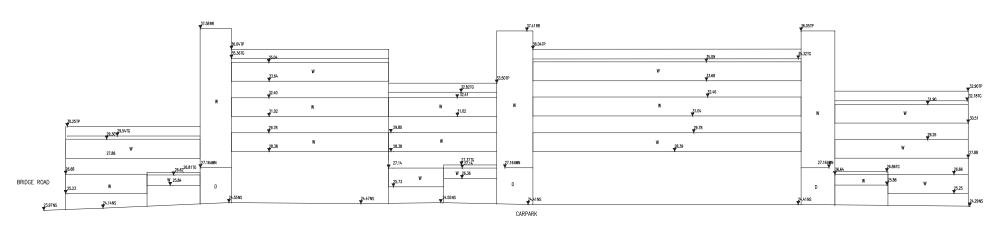




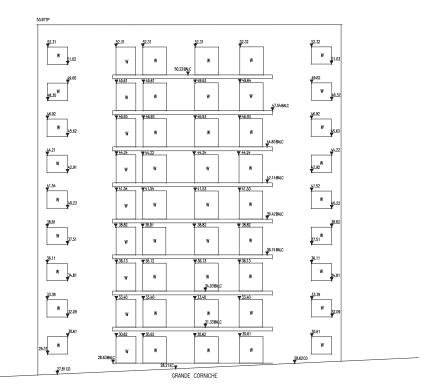


Appendix D Existing Site Survey

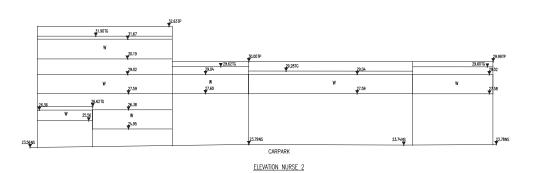


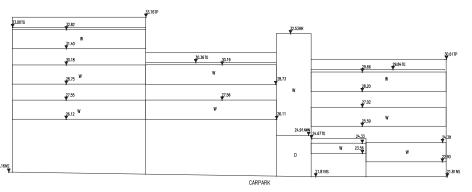


SOUTHERN ELEVATION WESTMEAD HOSPITAL ACCOMODATION 105 BRIDGE ROAD



NORTHERN ELEVATION 'MONARCO ESTATE'





ELEVATION NURSE 3

TYPICAL NO 1. ORG 2. BEAF	PM 1344, R.1.30.91 (A.H.D.) MGA NORTH	

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BCH	_	BOTTOM OF CHIMNEY	PERG	_	PERGOLA
BD	-	BOTTOM OF DOOR	PN	-	PERMANENT MARK
BOY	-	BOTTOM OF DOOR - SOUNDEY OF TOKICE BOTTOM OF BOX BOTTOM BOX	PP	-	POWER POLE
BF	-	BOTTOM OF FENCE	RCK	-	ROCK
BHR	-	BOTTOM OF HANDRAIL	FG,WY	-	RALWAY
BII	-	BITUMEN BOTTOM OF MEDD	100	=	POOC DIDCE
BI U	Ξ	BUILDING OF KEND	PCM	Ξ	CONCRETE NAI
RM	_	RENCH MARK	SEW	_	SEWER
BOL	_	BOLLARD	SHR	_	SHRUB
BR	-	BOTTOM OF ROCK	SIP	-	SEWER INSPECTION PIT
BRK	-	BRICK	SUH	-	SEWER LAMP HOLE
BRW	-	BOTTOM OF RETAINING WALL	SMH	-	SEWER MANHOLE
BS	-	BOTTOM OF STEPS	SP.	-	SIGN POST
RM	-	BOTTOM OF WALL	SSM	-	STATE SURVEY MARK
CUM	-	CURNICY WINDOW	SW SV	Ξ	STORM WATER
CI.	-	CENTRE INF	TARCH	ū	TOP OF ARCH
CLID	_	CONCRETE LID	TANN	`-	TOP OF AWNING
COL	-	COLUMN	TB	_	TOP OF BANK
COM	-	COMMUNICATIONS PIT	TBX	-	TELSTRA BOX
CON	- :	CONCRETE	TCH	-	TOP OF CHIMNEY
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D DD	-	DOOR DISH DRAIN	TEL	-	TELSTRA PIT
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DWY	_	DRIVEWAY	THR	_	TOP OF HANDRAI
EB	-	EDGE OF BITUMEN	n	_	TRAFFIC ISLAND
	-	ELECTRICITY BOX	TK	-	TOP OF KERB
EC	-	EDGE OF CONCRETE	TL.	-	TRAFFIC LIGHT
ED		EDGE OF DOOR	TP	-	TOP OF PARAPET
EG		EDGE OF GARDEN	TR	-	TREE
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GL	-	GROUND LEVEL	UKS	-	UNKNOWN SERVICE
GM	-	GAS METER	USB	-	UNDERSIDE OF CEILING
HPG HR	-	HANDRAIL	USC	Ξ	UNDERSIDE OF CHUNC
HMD		HYDRANT			UNDERSIDE OF GUTTER
L		INVERT LEVEL	v	-	VENT
ĸ	-	KERB	VER	_	VERANDAH
UN	-	LINTEL	VC	-	VEHICLE CROSSING
ሆ	-	LAMP POST	w	-	WINDOW
		WANHOLE	WM	-	WATER METER

LEGEND OF TYP	ICAL SERVICES		
— c —	COMMUNICATIONS CABLE		RECYCLED WATER
CSWP	CERAMIC STORMAKTER PIPE	RM	RISING MAIN
— E —	UNDERGROUND ELECTRICITY		ROADS & MARSTME SERV
	CABLES	— s —	SEWER
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rs	FIRE SERVICE	— sw —	STORM WIXTER
— c —	GAS LINE	—5WCK	STORMMATER CREEK
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FIG. 19 FASTE BEEK ST.

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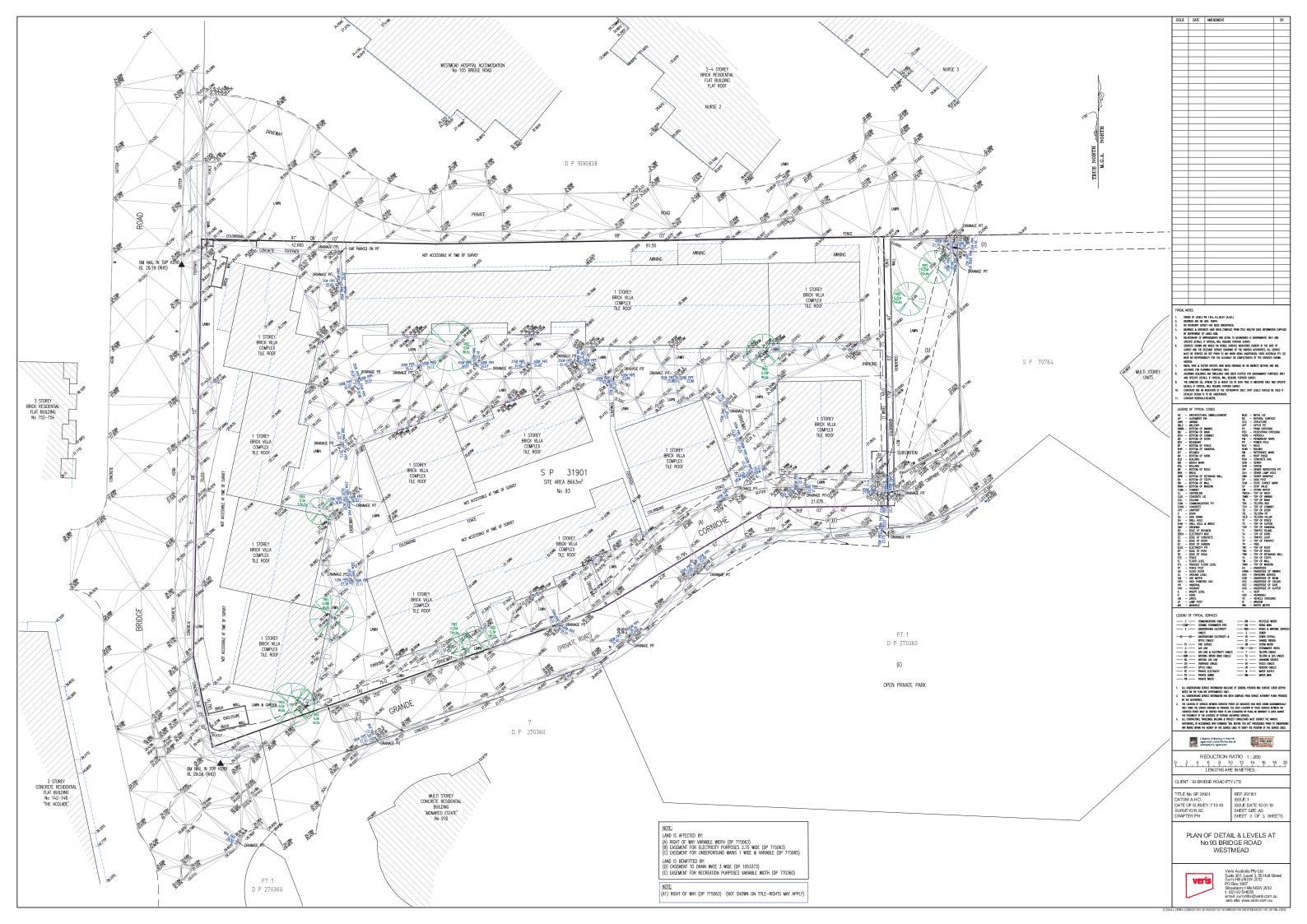
CLIENT: 93 BRIDGE ROAD PTY LTD

TITLE No.SP 31901 REF: 201161 ISSUE: 1
DATE OF SURREY.T.10.19 ISSUE DATE: 10.01.19 SHEET SURREY.SP SC DRAFTER: PH SHEET 2 OF 3 SHEETS

PLAN OF DETAIL & LEVELS AT No 93 BRIDGE ROAD WESTMEAD

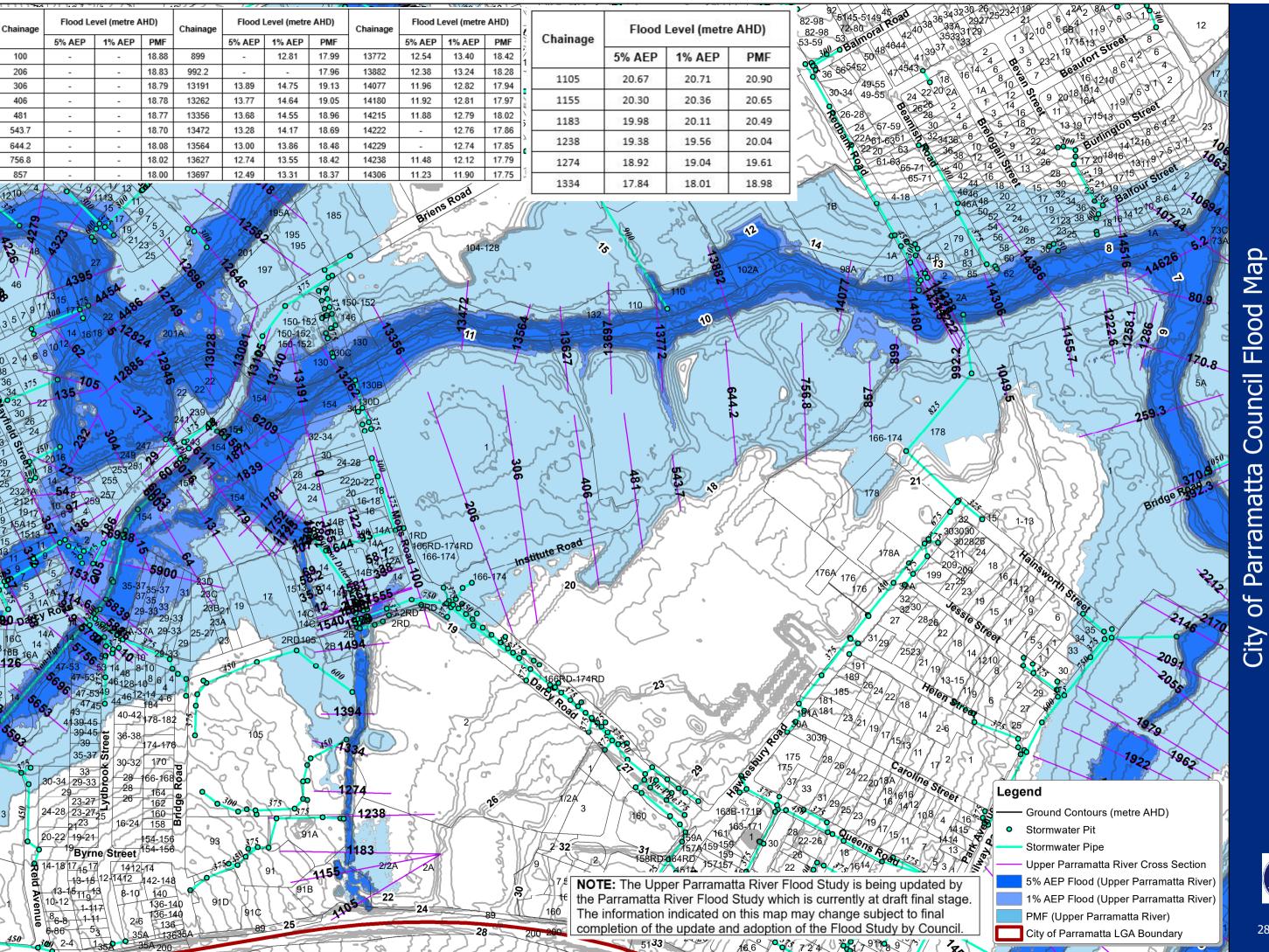


Veris Australia Pty Ltd Sulfe 301, Level 3, 55 Holt Street Surry Hills NSW 2012 PO Box 1907 Strawberny Hills NSW 2012 ti (02) 92124655 email: surryHills@wers com au web site: www.veris.com au





Appendix E Council Flood Information





Printed 28/02/2024